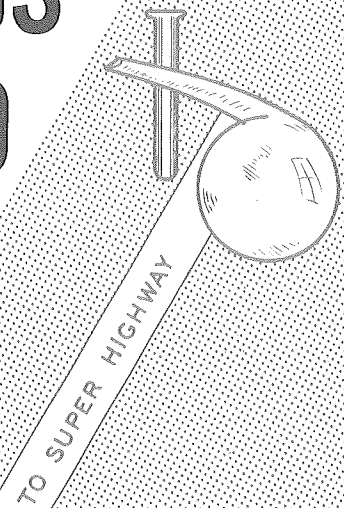


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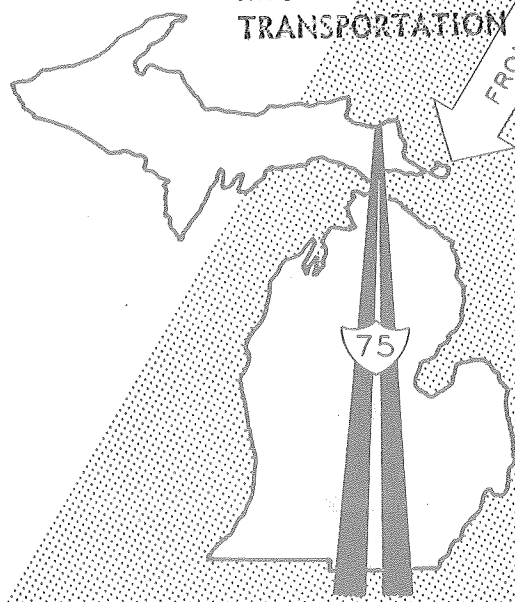
# MICHIGAN BITUMINOUS EXPERIMENTAL ROAD Final Report



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FEBRUARY 1967

Test Road personnel shown on the cover include (from left): Ward K. Parr, Laboratory Director, MDSH (Michigan Department of State Highways); Donald Arnott, Bituminous Plant Inspector, MDSH; Carl J. Stoike, Aggregate Inspector, MDSH; Arthur Hulkoff, Bituminous Plant Inspector, MDSH; John H. Goshorn, District Engineer, The Asphalt Institute; Arnold J. Hoiberg, Asphalt Research Engineer, Lion Oil Co.; Clifford J. Kole, Project Engineer, MDSH; William Donaldson, Assistant District Construction Engineer, MDSH; Owen Fisher, Contractor, Lake & Howell Construction Co.; Virgil Campbell, Truck Driver, Lake & Howell Construction Co.; Morley Durston, Superintendent, Lake & Howell Construction Co.; Edward Schwoppe, District Construction Engineer, MDSH; Edward Eriksen, Contractor, Lake & Howell Construction Co.; Paul J. Serafin, Bituminous Testing Engineer, MDSH; Tom Humphries, Bituminous Construction Engineer, MDSH.

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MICHIGAN BITUMINOUS EXPERIMENTAL ROAD: FINAL REPORT

Paul J. Serafin  
Larry L. Kole  
Alfred P. Chritz

Prepared for Presentation at the Annual Meeting of  
The Association of Asphalt Paving Technologists  
Denver, Colorado, February 13-15, 1967

Testing Laboratory Division  
Office of Testing and Research  
Report No. TB-15

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Michigan State Highway Commission  
Ardale W. Ferguson, Chairman; Charles H. Hewitt, Vice-Chairman;  
Wallace D. Nunn; Richard F. Vander Veen  
Lansing, February 1967

## INFORMATION RETRIEVAL DATA

REFERENCE: Serafin, Paul J. , Kole, Larry, L. , and Chritz, A. P. Michigan Bituminous Experimental Road: Final Report. Michigan Department of State Highways Departmental Report No. TB-15. February 1967.

ABSTRACT: This third and final report summarizes field and laboratory data covering 12 years of service of a test road which included asphalts from six sources, differing in viscosities, temperature susceptibilities, and heat stabilities. Coring, skid resistance tests, rut depth measurements, and visual observations are reported. Asphalt in two of the six sections of the project appeared to undergo more change with age, but performance generally justified use in regular construction of all asphalts involved.

KEY WORDS: asphalts, asphalt cements, crude oil, viscosity, plastic deformation, penetration, recovery, coring, experimental roads, pavement durability.

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## MICHIGAN BITUMINOUS EXPERIMENTAL ROAD: FINAL REPORT

### Background

In 1950 it appeared that several new sources of asphalt cement would become available to contractors in Michigan. At that time the Michigan Department of State Highways began an investigation to compare qualities of these new sources with others that had previously met specifications and given satisfactory service.

A laboratory study of 35 different asphalts, within penetration grades 60-70, 85-100, and 150-175, was completed in 1952 and a supplemental study of materials from Wyoming and Canadian crude oil sources completed in 1954. Reports on these studies (1, 2) indicated that some of the new sources had physical characteristics different from those with which satisfactory experience had been obtained. Three main physical characteristics were found to be different:

1. Viscosities,
2. Temperature susceptibilities, and
3. Heat stabilities.

It was felt that no definite conclusions regarding expected performance of these materials could be based on laboratory research. After consultation with industry and the Bureau of Public Roads it was decided to construct an experimental test road using various materials in an attempt to correlate actual construction and in-service performance with laboratory results.

### Project

The project identified as M 63-30, C8R selected for the test road was a 6-mile resurfacing of a 40-ft four-lane reinforced concrete pavement constructed in 1931. At the time of construction this was known as US 10 and was located in Oakland County between Pontiac and Flint. In 1962 a parallel route (I 75) was constructed, resulting in turning back this portion of the road to the Oakland County Road Commission. The project was

selected because this portion of roadway was heavily travelled and it was desired to subject the test road to fairly intensive traffic conditions in order to obtain performance information as quickly as possible.

Table 1 lists average daily traffic count by years during the service life of the test road. It is estimated that approximately 36,000,000 passenger and light commercial vehicles and 8,000,000 tractor-trailer units have passed over the test road since it was constructed. The decrease in traffic in 1958 is attributed to completion of US 23, a parallel expressway route, and further reduction in 1962 to the completion of I75, the parallel interstate route.

The test road portion of the project was constructed in six identical sections of 2400 ft each. The layout of individual sections and subsections was as shown in Figure 1.

TABLE 1  
AVERAGE  
DAILY TRAFFIC

Year	Average Daily Traffic	Percent Commercial
1954	11,000	22.3
1955	11,300	22.1
1956	11,800	21.6
1957	12,200	21.3
1958	10,800	17.5
1959	10,900	17.0
1960	11,000	17.3
1961	11,500	17.3
1962	7,700	14.3
1963	8,300	13.3
1964	7,300	13.7
1965	7,300	13.7
1966	7,300	13.7

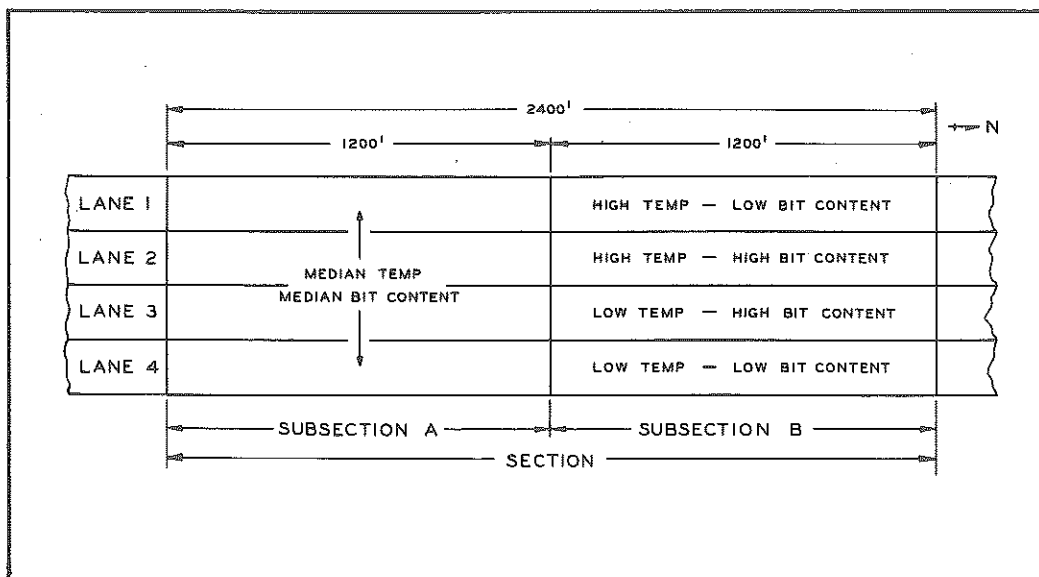


Figure 1. Typical layout of test road sections and subsections.

## Materials

The contractor was required to use the same sources of aggregates in producing the bituminous mixtures for each of the test sections. In addition, mixture proportions were maintained as uniform as possible in order to eliminate any effects such variations would have on pavement performance or physical properties of the asphalt or mixtures. Table 2 lists sources and proportions of materials used.

TABLE 2  
WEARING COURSE BITUMINOUS MIXTURE  
MATERIALS AND PROPORTIONS

Material	Source	Typical Batch Proportioning, percent
25A Coarse Aggregate	American Aggregate Corp., Green Oak, Mich.	55.0
3BC Fine Aggregate	Blend of two local sands	34.5
3MF Limestone Mineral Filler	National Lime & Stone Co., Findlay, Ohio	5.0
60-70 Asphalt Cement	{ Section 1-Wyoming Crude, Refinery A Section 2-Venezuelan Crude Section 3-Wyoming Crude, Refinery B Section 4-West Texas Winkler Crude Section 5-Arkansas Smackover Crude Section 6-East Texas Talco Crude }	5.5

The bitumen contents used in producing the mixtures were determined from results of Marshall check tests conducted on two sources of asphalt whose viscosity characteristics were the outer limits of the ranges studied. As shown in Figure 1, the high, low, and median bitumen content mixes were produced and placed at high, low, and median temperatures at specified locations in each test section. These variations--bitumen content and temperature--were the only differences programmed for production and placing of the mixtures. All other production and construction operations were controlled as uniformly as was practical.

The high, low, and median temperatures were determined from viscosity characteristics of the asphalts. Table 3 lists results of various consistency tests performed on the test road asphalts. The 1954 data are results obtained on fresh asphalt samples taken prior to construction. The 1965 data are results obtained after bulk storage for 11 years in 5-gal containers. These were heated and their entire contents blended together for testing. The surface is believed to have hardened during storage, and blending this into the balance of the sample may have contributed to the



changes in consistency noted. Figure 2 shows viscosity-temperature curves of the original asphalts taken before start of construction. An attempt was made during construction to maintain mixture temperatures as uniform as possible so that the asphalts used would have Saybolt Furol viscosities of 75 sec at high temperatures and 200 sec at low temperatures. The median temperatures used were generally close to the average of the high and low temperatures for each type of asphalt. Although temperatures were maintained fairly uniformly, it was not always possible to obtain the desired viscosities due to problems of moisture removal and workability. Table 4 lists average mix temperatures at the street observed in paving each section and corresponding asphalt viscosities.

TABLE 3  
RESULTS OF CONSISTENCY TESTS ON  
ORIGINAL TEST ROAD ASPHALTS\*

Test	Section 1		Section 2		Section 3		Section 4		Section 5		Section 6	
	1954	1965	1954	1965	1954	1965	1954	1965	1954	1965	1954	1965
<u>Penetration</u>												
59 F, 100 g, 5 sec, dmm	22		21		22		23		21		23	
77 F, 100 g, 5 sec, dmm	63	58	60	57	67	58	60	53	61	52	65	61
95 F, 100 g, 5 sec, dmm	173		154		186		151		144		141	
<u>Softening Point (R &amp; B), F</u>												
	120		125		119		123		120		124	
<u>Viscosity</u>												
<u>Saybolt Furol</u>												
225 F, centistokes	192	211	313	338	197	204	217	237	276	288	337	345
300 F, centistokes	101		170		105		128		150		172	
325 F, centistokes	61		100		61		84		91		93	
<u>Kinematic</u>												
140 F, stokes		2457		4336		2361		3159		3869		4418
275 F, centistokes		439		676		399		454		576		675

\*1954 - Tests run on original asphalts before test road was paved.

1965 - Samples of the original asphalts were taken in 1954 and stored in 5-gallon cans.

### Construction Procedure

The project was scheduled for construction during summer months to take advantage of optimum Michigan weather conditions. Since special equipment and operations were required, to maintain uniform temperatures and quality of mixtures produced, 1-mile organizational sections at each end of the project were paved first to familiarize personnel with operations and equipment before work started on the test sections.

Binder course mixture was placed at the rate of 170 lb per sq yd over each test section, immediately followed by the placing of wearing course mixture in the pattern shown in Figure 1 at the rate of 130 lb per sq yd. Construction time required to complete each 1/2-mile test section of 40-ft pavement was 2-1/2 to 3 days. The same asphalt source was used for both binder and wearing course in each test section.

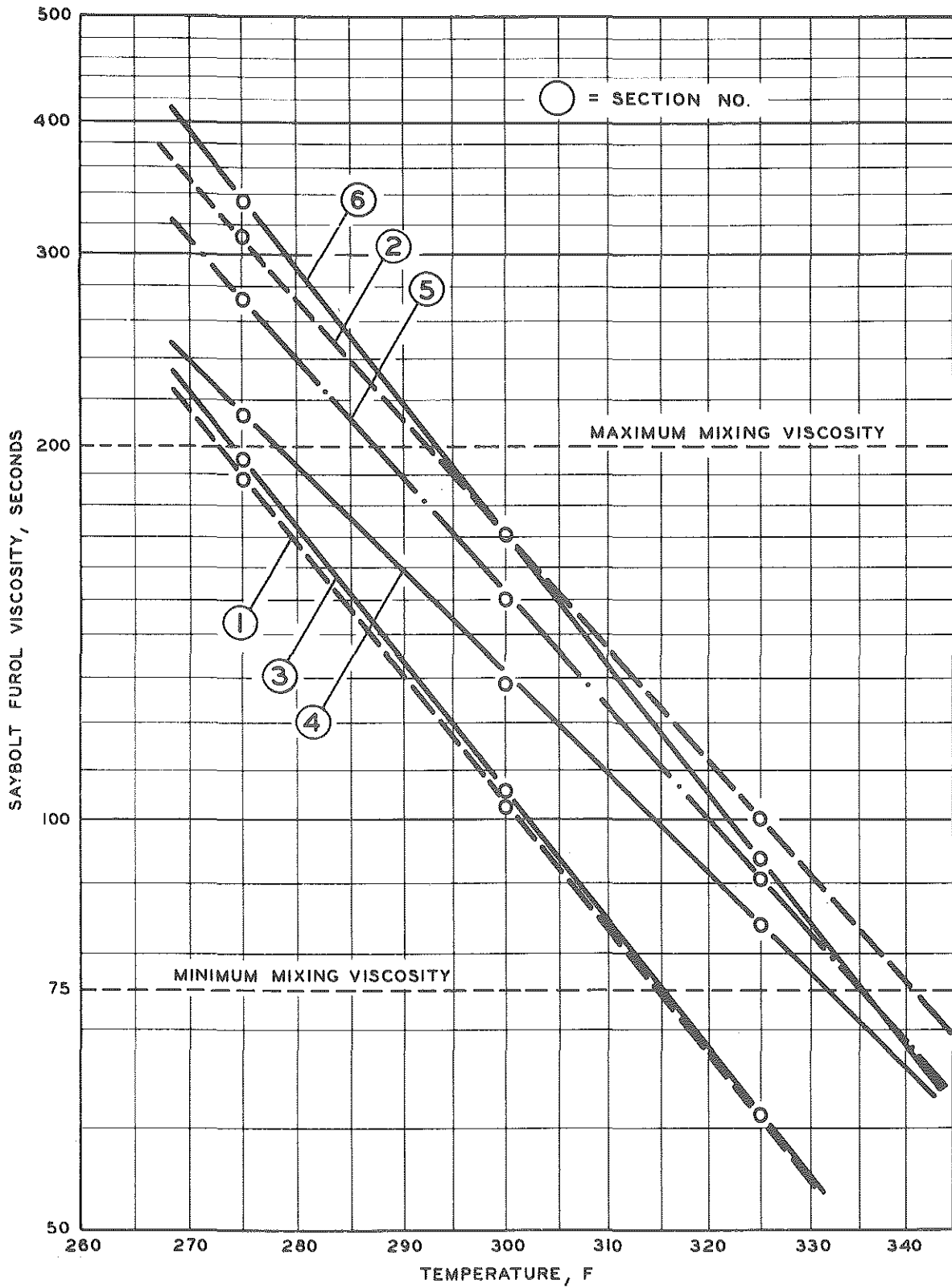


Figure 2. Viscosity-temperature curves of asphalts used for indicated test sections.

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TABLE 4  
AVERAGE TEMPERATURES  
OF BITUMINOUS CONCRETE WEARING COURSE MIXTURES  
AND CORRESPONDING ASPHALT CEMENT VISCOSITIES

Sub-Section	Lane	Street Temperature, F	Saybolt-Furol Viscosity, sec.*	Sub-Section	Lane	Street Temperature, F	Saybolt-Furol Viscosity, sec.*
1A	1	300	103	4A	1	295	144
	2	295	118		2	295	144
	3	295	118		3	300	129
	4	285	148		4	300	129
1B	1	310	82	4B	1	265	266
	2	310	82		2	285	175
	3	285	148		3	310	109
	4	285	148		4	315	98
2A	1	310	138	5A	1	305	135
	2	300	170		2	295	171
	3	320	112		3	310	122
	4	325	100		4	305	135
2B	1	335	83	5B	1	320	99
	2	325	100		2	320	99
	3	290	215		3	295	171
	4	290	215		4	285	212
3A	1	300	107	6A	1	295	193
	2	310	84		2	295	193
	3	310	84		3	290	220
	4	300	107		4	300	172
3B	1	315	76	6B	1	320	105
	2	305	94		2	320	105
	3	285	171		3	290	220
	4	285	171		4	290	220

\* Saybolt Furol viscosities at street temperatures interpolated from viscosity curves.

## FIELD AND LABORATORY OBSERVATIONS

### Field Data Collection

At the time of construction, temperatures of each truckload were measured at the plant and paver. Two truckloads from each lane in each subsection were sampled to supply mixture for Marshall specimens. These Marshall specimens were tested to determine their properties, and a mix analysis and recovery test were run. Samples of pavement produced from the same truckloads were taken for determination of physical properties, mix analysis, and recovery tests.

In November 1954, the test road coring program began. Through 1957 at least four core samples at two to four locations in each subsection were obtained annually for laboratory analysis. In December 1957, the number of core samples was increased so that both the center and one wheel track of each lane could be analyzed. Figure 3 shows a typical core location. Approximately 2880 core samples have been obtained from the test road since it was completed. Core test results are summarized in Appendices A and B.



Figure 3. Typical core location.

Skid resistance tests were conducted on the test road in 1958 and 1963, with the results given in Table 5. By 1958, this bituminous surface had already been subjected to 4 years of polishing action by traffic.

The coarse aggregate used was a predominantly silicious crushed gravel. With this type of aggregate, not much additional polishing was expected to take place after 1963. This is substantiated by Table 5, which indicates only a slight decrease in average coefficients between 1958 and 1963. The lack of any significant difference of coefficient of friction in the various sections and lanes indicates uniformity of the pavement surfaces and the absence of any effect of asphalt source on this property. The difference in values between traffic and passing lanes indicates effects of both number of repetitions of wheel passages and higher wheel loadings in the traffic lane.

TABLE 5  
SKID RESISTANCE TEST RESULTS  
Coefficients of Wet Sliding Friction at 40 mph

Section	Traffic Lanes		Passing Lanes	
	May 1958	July 1963	May 1958	July 1963
1	0.43	0.42	0.51	0.44
2	0.44	0.44	0.50	0.48
3	0.43	0.44	0.50	0.48
4	0.40	0.42	0.51	0.47
5	0.42	0.41	0.49	0.46
6	0.41	0.40	0.49	0.47
Avg.	0.42	0.42	0.50	0.47

Visual observations have been made at periodic intervals. No serious deterioration has been observed in any of the test sections, although all sections have developed fairly extensive reflective cracking. Figure 4 shows pavement condition prior to resurfacing and after 12 years of service. Some differences in coloration do appear between sections, but not to any degree that can be correlated with the core data obtained.

During August 1966, rut depth measurements were made at 100-ft intervals in each lane of each test section (Appendix C). Figure 5 shows the 10-ft straight-edge used in measuring maximum rut depth in each wheel track. It was felt that such measurements might provide data that could be correlated with core information or with the horizontal or vertical alignments of the test sections.

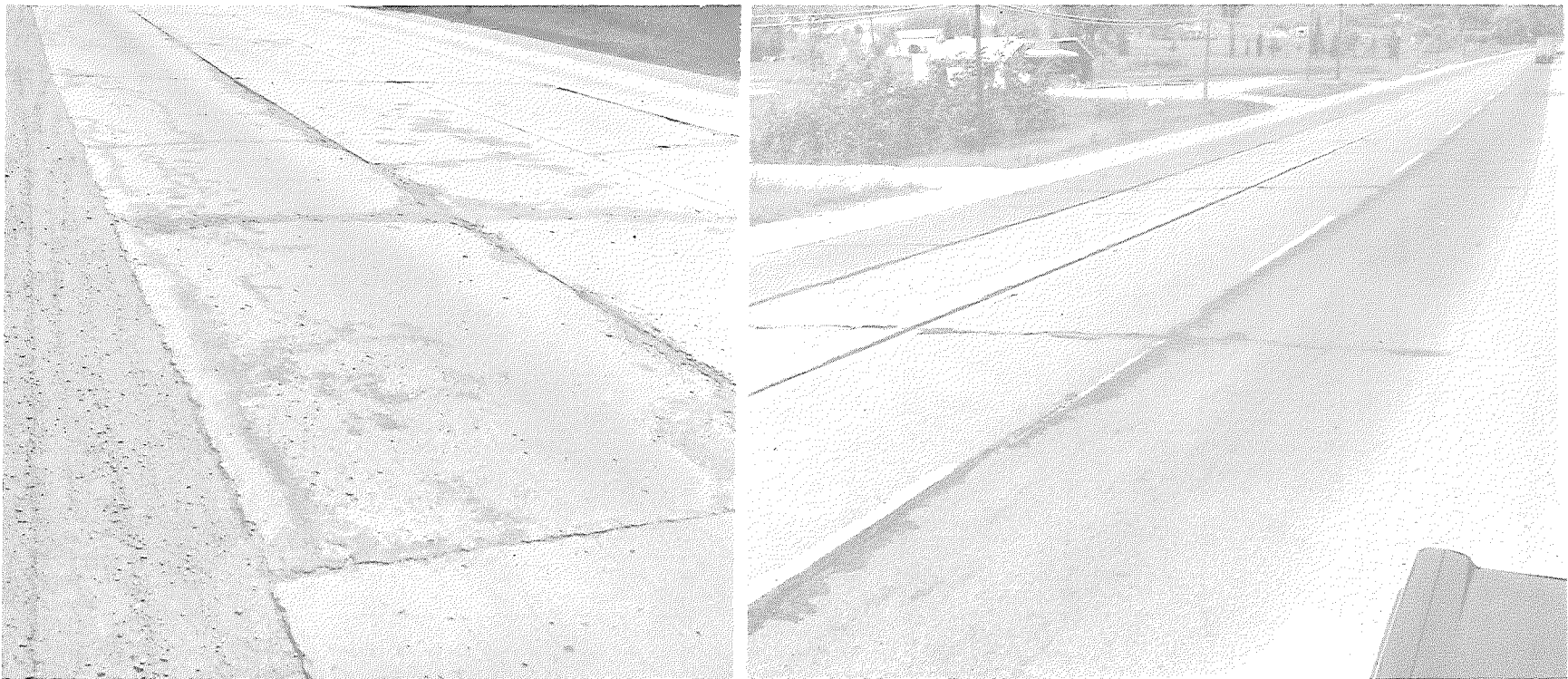


Figure 4 (above). Appearance of concrete before re-surfacing (left) and test road surface after 12 years of service (right).

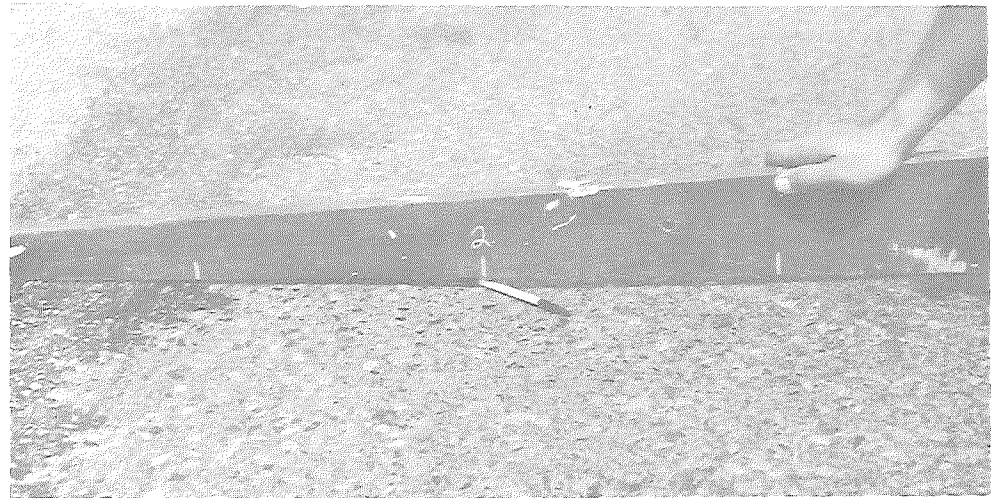


Figure 5 (right). Rut depth measurement device.

## Laboratory Physical and Chemical Analyses

A solvent separation into major constituents was made on the six test road asphalts as recovered from pavement cores after service of approximately 3-1/2 years. In addition, for comparative purposes a solvent separation was also made on the six test road asphalts set aside during construction in 1954 and stored in the laboratory in 5-gal pails for the same 3-1/2-year period.

The Abson method was used to recover the asphalt from the pavement cores following the procedure described in ASTM Method D 1856. The asphalts were separated quantitatively into four major constituents: asphaltenes, oily constituents, light resins, and heavy resins. In addition, to determine the degree of saturation of the different components, an Iodine Number was determined for each. The results of these analyses did not correlate significantly with service performance of these six asphalts (Table 6).

The procedure used for the solvent separations and method used to determine the Iodine Number are summarized as follows:

Asphaltenes. The asphalt was dispersed in 86 deg Baumé paraffin naphtha and the precipitate filtered through a tared Gooch crucible having an asbestos mat. The tared crucible containing the precipitate was dried at 100 C and weighed to determine the percentage of asphaltenes.

Oily Constituents. The filtrate from the asphaltene procedure was poured into a paper-type Soxhlet thimble containing fuller's earth and refluxed with 86 deg BÉ naphtha. The extracted naphtha was then evaporated, and the residue dried at 65 C and weighed to determine the percentage of oily constituents.

Light Resins. The Soxhlet thimble containing fuller's earth and the balance of the asphaltic components was then refluxed with benzene. The extract was evaporated, and the residue dried at 100 C and weighed to determine the percentage of light resins.

Heavy Resins. The Soxhlet thimble containing fuller's earth and the remaining asphaltic components was then refluxed with acetone. The extract was evaporated, and the residue dried at 100 C and weighed to determine the percentage of heavy resins.

Iodine Number. The Iodine Number was determined using the Wijs reagent method.

TABLE 6  
PHYSICAL AND CHEMICAL ANALYSES OF ASPHALT CEMENTS

Section →	1		2		3		4		5		6	
Crude Source →	Wyoming, Refinery A		Venezuelan		Wyoming, Refinery B		West Texas Winkler		Arkansas Smackover		East Texas Talco	
Asphalt Cement →	Original	Recovered	Original	Recovered	Original	Recovered	Original	Recovered	Original	Recovered	Original	Recovered
Date Sampled	1954	1957	1954	1957	1954	1957	1954	1957	1954	1957	1954	1957
Penetration	63	37	60	37	67	30	60	41	61	37	65	36
Asphaltenes, %	19.1	20.6	24.3	26.3	18.4	20.6	17.2	19.4	19.0	21.3	25.4	28.6
Iodine No.	67.6	57.6	75.9	68.2	71.3	63.3	60.0	57.3	67.0	63.9	72.8	64.6
Oily Constituents, %	42.4	42.6	40.4	32.1	43.8	38.2	40.9	39.3	38.4	37.4	39.2	38.6
Iodine No.	51.0	48.2	70.4	64.0	51.0	38.0	39.6	35.2	37.6	34.7	60.6	45.4
Light Resins, %	17.8	11.8	13.5	18.0	13.9	17.7	16.7	20.2	19.4	18.0	12.5	12.3
Iodine No.	98.4	88.9	128.7	75.8	110.4	70.7	80.6	76.2	97.7	64.2	85.0	60.1
Heavy Resins, %	18.9	23.5	21.8	23.6	23.7	23.0	24.6	19.7	22.4	21.8	21.0	18.8
Iodine No.	<u>72.4</u>	<u>60.7</u>	<u>95.1</u>	<u>65.1</u>	<u>78.9</u>	<u>61.5</u>	<u>49.3</u>	<u>59.5</u>	<u>96.0</u>	<u>60.8</u>	<u>71.7</u>	<u>72.5</u>
Total, %	98.2	98.5	100.0	100.0	99.8	99.5	99.4	98.6	99.2	98.5	98.1	98.3



## DISCUSSION

Prior reports on the bituminous test road (3, 4) indicated certain features considered pertinent in 1955 and 1959. After 12 years of service, the trends observed earlier have been confirmed by the additional data obtained.

In an attempt to determine how well the various test sections performed, statistical analyses were undertaken. These showed a wide dispersion or variation in quantitative values obtained for the various properties of the asphalt cements used and pavement densities, regardless of the manner in which the data were grouped. Indeed, variations were expected to result from service time or aging effects. The analyses did show a wider variation in data for asphalt properties obtained from Sections 2 and 6, indicating that the asphalts used there underwent more change than other asphalts.

TABLE 7  
STATISTICAL FUNCTIONS FOR DATA  
FROM 570 EXTRACTION TESTS  
PERFORMED ON PAVEMENT CORE SAMPLES  
OF WEARING COURSE MIXTURE

	Arithmetic Mean, %	Standard Deviation, %
	$\bar{X}$	$\sigma$
<u>Mixture Proportions</u>		
Coarse Aggregate (R10)	57.33	1.21
Fine Aggregate (P10-R200)	32.04	1.14
Dust (P200)	5.48	0.60
Bitumen	5.17	0.21
<u>Gradation of Extracted Aggregate</u>		
Passing 5/8 in.	99.98	0.14
Passing 1/2 in.	97.35	1.41
Passing 3/8 in.*	80.09	2.55
Passing No. 4	50.25	1.94
Passing No. 10	39.57	1.26
Passing No. 40	30.78	1.56
Passing No. 80	15.46	1.09
Passing No. 200	5.77	0.63

\*3/8 in. sieve was added in 1961 and only 246 extraction tests were performed using this sieve.

Table 7 is a compilation of statistical functions for results obtained from the 570 extraction tests performed on pavement core samples of wearing course mixture. It is interesting to note that there is very little

difference in standard deviations for each sieve size analyzed in the gradation of extracted aggregate, except material passing the No. 200 sieve. This is borne out in the analysis of mixture proportions, which shows a higher standard deviation for the mineral filler in proportion to the quantity evaluated than for the other ingredients. Since a fairly uniform quantity of mineral filler was added to the mixture during production, and there is no evidence of degradation in the aggregates used, this indicates that there may have been some fluctuation in this fraction present in the raw materials.

Figures 6 and 7 are plots of specific gravities and penetrations of recovered asphalt expressed as a percentage of original penetration versus length of pavement service in months. These plots of data from wheel track pavement cores indicate an increase in pavement density up to some maximum value and then very little change. The penetration values show a decrease, but the rate of decrease generally becomes smaller as the pavement ages. The wheel track data were used for these plots because of fluctuations present in data obtained from center-of-lane core samples. These fluctuations are partially attributed to oil drippings and other road debris impregnating the wearing course.

Regression analyses were performed on the data used for Figures 6 and 7. Equations developed from these analyses were of the following form:

$$\begin{aligned} \text{Specific Gravity} &= a + b (\ln t) \\ \text{Percent of Original Penetration} &= c - dt \end{aligned}$$

where

$$\begin{aligned} a, b, c, \text{ and } d &= \text{constants} \\ t &= \text{test road service life in months.} \end{aligned}$$

Correlation coefficients (r values) for the data analyzed are listed and indicate much better correlation for the specific gravity data. It is interesting to note that correlation coefficients for traffic lane specific gravity data were generally higher than passing lane data.

Table 8 presents average values determined from maximum rut depths measured with a straight edge transversely across the pavement. Plan and profile views of the test sections are shown in Figure 8. As can be seen, there is very little significant difference between various sections in results obtained for a given lane. It should be noted that much more

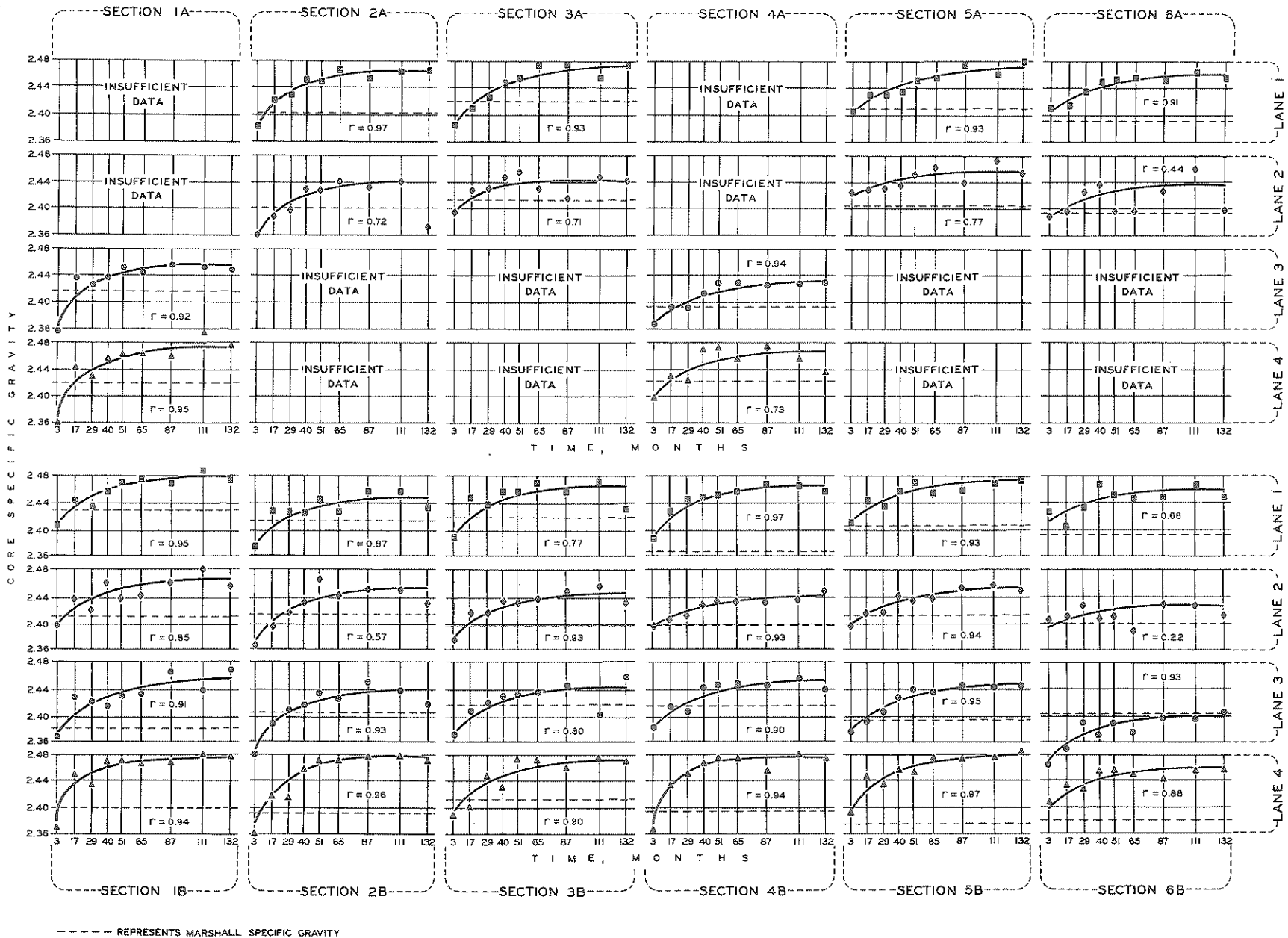


Figure 6. Change in specific gravity of wheel track core samples with time.

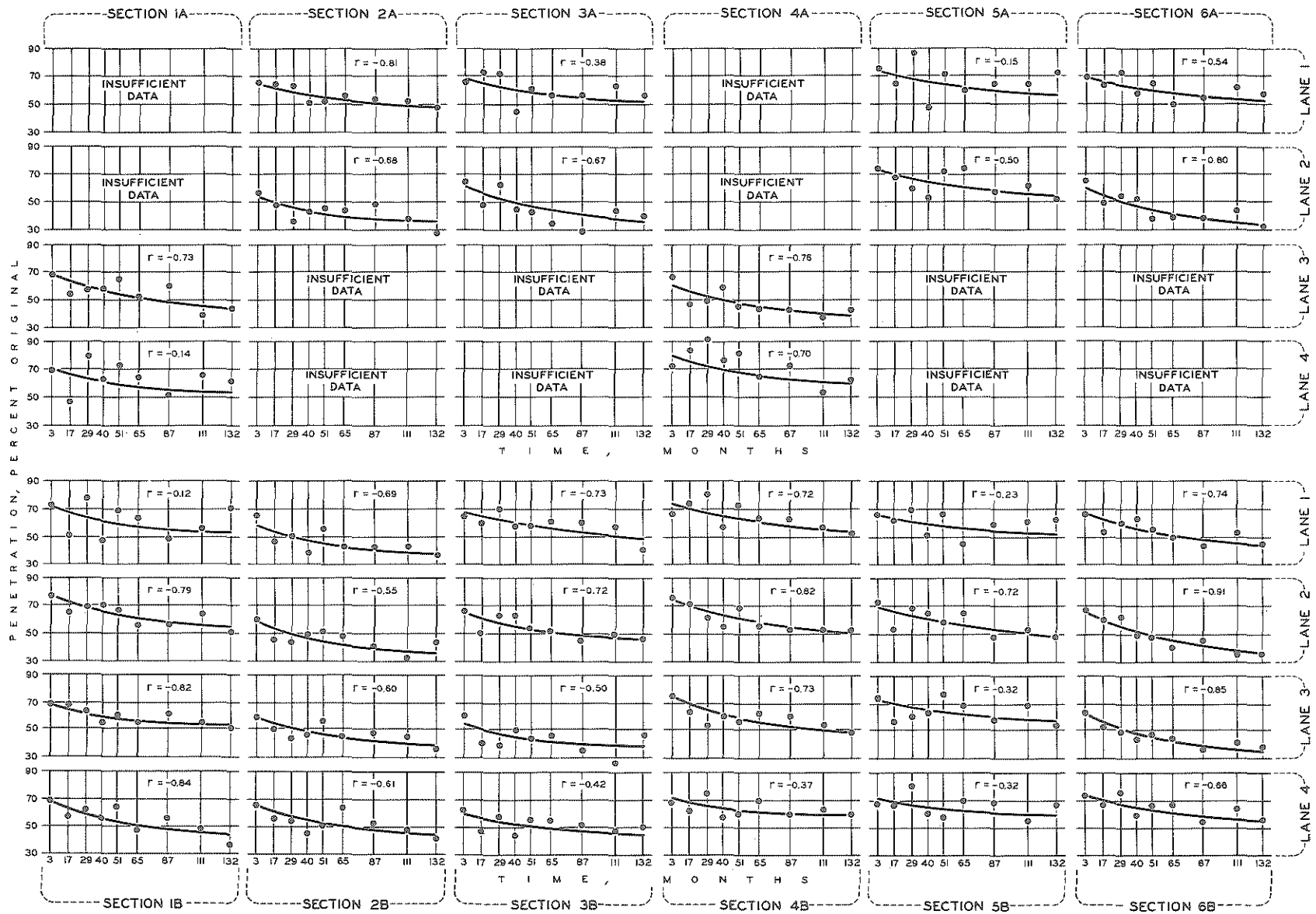


Figure 7. Change in penetration of recovered asphalt from wheel track core samples with time.

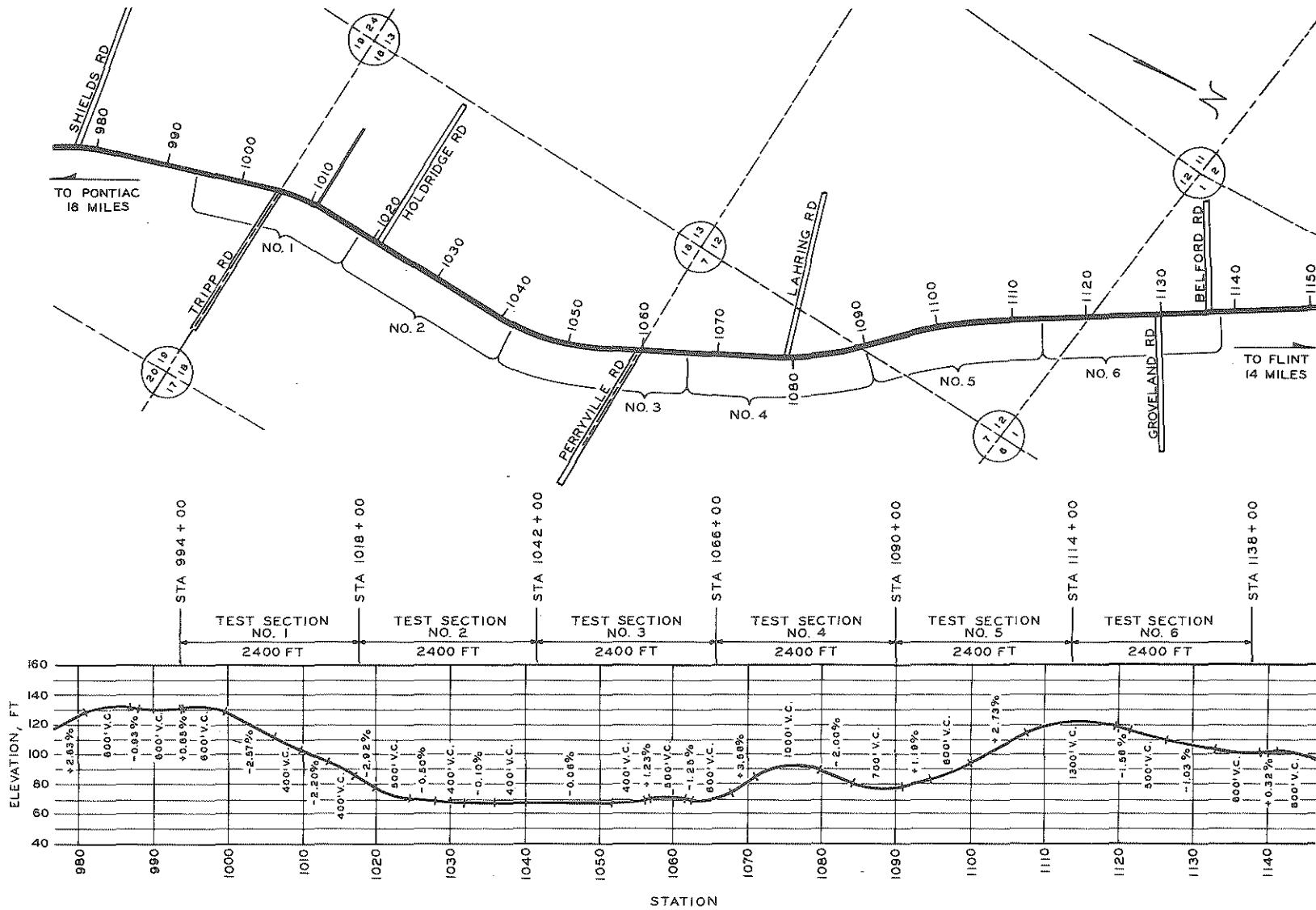


Figure 8. Plan and profile of the test road.

rutting developed in traffic lanes than passing lanes. Analysis of the data in Appendix C indicated no significant correlation between rutting and vertical alignment. In general, the greatest rutting occurred in the outermost lane around horizontal curves, indicating that a component of centrifugal force contributed to an increased force applied normal to the pavement surface in these areas. Another significant indication is that more rutting occurred in the northbound traffic lane than the southbound. It is believed that this is due to northbound truck traffic being more heavily loaded, because of numerous gravel pits at the south end of the test project.

TABLE 8  
SUMMARY OF RUT DEPTH MEASUREMENTS

Lane	Wheel Track	Average Rut Depth for Subsection Indicated, in.											
		1A	1B	2A	2B	3A	3B	4A	4B	5A	5B	6A	6B
1	Right	0.16	0.08	0.14	0.16	0.20	0.21	0.14	0.18	0.19	0.15	0.13	0.18
	Left	0.11	0.05	0.15	0.16	0.20	0.15	0.22	0.15	0.10	0.12	0.11	0.11
2	Right	0.03	0.02	0.05	0.06	0.03	0.05	0.09	0.06	0.03	0.06	0.05	0.03
	Left	0.07	0.04	0.09	0.03	0.04	0.08	0.12	0.09	0.07	0.09	0.09	0.06
3	Left	0.01	0.03	0.03	0.05	0.04	0.06	0.04	0.04	0.04	0.05	0.05	0.03
	Right	0.02	0.01	0.03	0.03	0.07	0.05	0.05	0.02	0.03	0.06	0.03	0.06
4	Left	0.16	0.21	0.13	0.11	0.07	0.17	0.28	0.15	0.18	0.38	0.16	0.23
	Right	0.19	0.19	0.28	0.22	0.19	0.27	0.36	0.21	0.23	0.24	0.17	0.22

No correlation could be established regarding the relation of increase in rut depth to decrease in asphalt viscosity, as was found by Speer (5). Lack of such correlation may be due to Speer's better precision of measurement, as well as other effects not as controllable in a field experiment as in his controlled laboratory study.

#### Summary

Based on service behavior of the asphalts and information in previously published reports (3, 4) the asphalt cements from those sources used on the test road were approved for bituminous construction on State trunklines in Michigan.

One of the test road's objectives was to determine if any construction problems would arise from use of new asphalt sources becoming available in Michigan. No problems arose and subsequent performance of the test road has provided justification for permitting their use since 1955. In

addition, it soon became apparent that the voluminous data gathered from this project were an excellent source of valuable information of interest to the bituminous industry. This information is presented either in the text of this report or its Appendices, with discussion of those subjects of interest to Michigan, while other data have been left in raw form and are available for further evaluation by others.

Results of tests on recovered asphalts from the various test sections show more marked changes in the properties of asphalts from Sections 2 and 6. One might expect these changes to be reflected in performance of the six sections. However, with many engineers inspecting the test road during the past 12 years, there have been some differences of opinion, in which some felt that slightly more cracking had developed in Sections 2 and 6 than in the balance of the project. These conditions may or may not correlate with some physical changes in the asphalt cements observable in the data presented in this report. Since there may have been variations in condition of the concrete pavement at the time of resurfacing as well as in the underlying base materials, it would be difficult to make any firm, significant conclusions regarding any differences in condition of the six test sections.

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APPENDIX A

WEARING COURSE

Summary of Laboratory Analyses of Bituminous Concrete

- Section 1. Wyoming Crude, Refinery A
- Section 2. Venezuelan Crude
- Section 3. Wyoming Crude, Refinery B
- Section 4. West Texas Winkler Crude
- Section 5. Arkansas Smackover Crude
- Section 6. East Texas Talco Crude





## SECTION 2. VENEZUELAN CRUDE

Location Sampled (b)	Date Sampled	Mixture Analysis, %				Gradation of Extracted Aggregate % passing indicated sieve										Recovered Asphalt			Measured Specific Gravity (d)	% Voids	% Filled With Bitumen	% Voids in Mineral Aggregate	Compaction %
		R 10	P 10 R 200	P 200	Bitumen	5/8	1/2	3/8	4	10	40	80	200	Penetration (c)	Ductility	Ash, %							
Lane 1 (a)	M	8-54	54.2	33.7	6.7	5.4	100	98	52	43	33	16	7.1	40	150+	2.7	2.415	4.8	73	17.8	100.0		
	--	11-54	58.1	32.2	6.5	5.2	100	97	52	41	32	15	6.8	41	150+	0.2	2.383	6.1	67	18.5	98.8		
	--	1-56	55.9	31.8	7.0	5.6	100	98	51	41	32	16	7.4	40	150+	2.3	2.421	4.6	74	17.7	100.2		
	--	1-57	58.1	36.8	5.8	5.7	100	98	50	38	29	14	5.9	39	150+	0.1	2.430	4.3	76	17.9	100.6		
	WT	12-57	58.7	39.9	5.9	5.1	100	95	47	38	30	18	6.2	31	150+	2.1	2.452	3.4	78	16.5	101.5		
	C	12-57	59.2	30.4	5.2	5.2	100	98	49	39	30	17	5.6	34	150+	0.8	2.431	4.2	75	19.8	100.5		
	WT	11-58	58.2	31.0	6.7	6.1	100	98	48	39	29	15	6.0	32	150+	2.3	2.451	3.4	78	16.5	101.4		
	C	11-58	57.3	31.5	5.7	5.5	100	98	50	39	30	16	6.0	41	150+	0.5	2.432	4.2	78	17.6	100.7		
	WT	1-60	58.5	30.6	5.6	5.3	100	98	48	38	29	15	5.8	35	150+	0.4	2.468	2.8	82	16.8	102.2		
	C	1-60	58.2	31.1	5.3	5.4	100	98	49	38	29	15	5.8	38	150+	0.7	2.444	3.7	78	16.5	101.2		
	WT	11-61	58.4	31.8	4.6	5.2	100	98	54	52	39	31	15	4.9	33	150+	2.7	2.454	3.3	79	16.7	101.6	
	C	11-61	57.7	32.2	4.7	5.4	100	98	55	53	39	32	17	5.0	38	147	1.5	2.432	4.2	75	16.8	100.7	
Lane 2	M	8-54	56.2	32.8	5.7	5.3	100	99	52	41	32	16	6.0	49	150+	2.1	2.414	4.9	72	17.5	100.0		
	--	11-54	54.8	33.3	6.8	5.1	100	97	54	42	34	16	7.2	35	150+	0.3	2.390	7.0	63	18.9	97.8		
	--	1-56	57.1	32.5	5.5	4.9	100	97	51	40	32	15	5.8	29	150+	1.6	2.417	5.6	66	17.4	98.9		
	--	1-57	56.5	31.4	6.7	5.4	100	95	56	40	32	17	7.1	22	120+	0.3	2.398	5.5	70	18.3	99.3		
	WT	12-57	56.5	32.4	5.9	5.2	100	99	52	40	32	17	6.2	26	150+	1.0	2.428	4.3	74	16.5	100.8		
	C	12-57	57.1	32.1	5.8	5.0	100	98	57	40	32	17	6.1	28	150+	0.6	2.422	4.0	72	16.4	100.3		
	WT	11-58	58.4	30.9	5.7	5.0	100	90	49	38	30	16	6.0	28	150+	0.4	2.428	4.4	73	16.3	100.5		
	WT	1-60	58.6	31.0	6.3	5.1	100	97	51	38	30	16	5.8	28	150+	0.7	2.440	3.5	78	15.9	101.1		
	WT	11-61	58.5	30.9	5.1	5.3	100	95	50	37	28	14	5.8	30	115	1.4	2.402	4.2	74	16.2	100.7		
	WT	11-63	59.0	30.3	5.8	4.9	100	97	51	50	38	30	16	6.1	23	119	1.3	2.440	3.9	75	15.8	101.1	
	C	11-63	57.7	31.0	5.5	5.2	100	97	78	51	39	30	16	5.8	29	160+	2.1	2.394	6.1	66	17.9	98.8	
	WT	8-65	58.6	33.0	5.2	5.2	100	99	63	51	40	30	16	5.5	17	46	1.4	2.398	5.9	67	17.9	98.9	
C	8-65	58.0	31.4	5.5	5.1	100	97	77	49	39	30	15	5.8	28	150+	2.3	2.432	4.2	74	16.2	100.7		
Lane 3	WT	11-63	58.1	30.9	6.2	4.8	100	97	79	49	39	32	17	6.5	19	75	1.0	2.405	5.2	68	16.3		
	C	11-63	58.8	31.4	5.0	4.8	100	98	78	48	38	30	15	5.3	20	77	1.3	2.431	4.2	73	15.6		
	WT	8-65	57.4	32.0	5.4	5.2	100	97	78	49	39	31	16	5.7	28	150+	1.8	2.451	3.4	78	15.5		
	C	8-65	57.9	31.8	5.2	5.1	100	96	73	49	38	31	17	5.6	19	97	1.9	2.422	4.6	72	16.4		
Lane 4	WT	11-63	58.3	31.1	5.4	5.2	100	97	79	51	39	30	18	5.7	35	150+	1.1	2.454	3.3	79	15.7		
	C	11-63	54.8	33.7	6.1	5.4	100	94	56	42	35	17	6.4	29	150+	2.0	2.451	3.4	79	18.2			
	WT	8-65	58.3	32.5	4.1	5.1	100	97	80	51	39	29	14	4.3	38	150+	1.4	2.465	2.9	81	15.3		
	C	8-65	58.3	31.8	4.7	5.2	100	97	78	61	38	29	14	5.0	34	150+	1.4	2.438	3.9	76	16.2		

(a) Lanes numbered west to east.

(b) M = mix sample, C = core from center of lane, WT = core from wheel track. Dashes indicate cores taken at random locations.

(c) Penetrations of original asphalt = 60 (Section 2B Lanes 1, 3, 4) / 61 (Section 2A all lanes; Section 2B Lane 2)

(d) Maximum theoretical specific gravities = 2.538 (Section 2A all lanes) / 2.541 (Section 2B Lanes 2, 3) / 2.548 (Section 2B Lanes 1, 4)







# SECTION 6. EAST TEXAS TALCO CRUDE

Location Sampled (a)	Date Sampled	Mixture Analysis, %						Gradation of Extracted Aggregate % passing indicated sieve								Recovered Asphalt			Measured Specific Gravity (c)	% Volids	% Volids Filled With Bitumen	% Volids in Mineral Aggregate	Compaction %					
		R 10	P 10 R 200	P 200	Bitumen	6/8	1/2	3/8	4	10	40	80	200	Penetration (c)	Ductility	Ash, %												
Lane 1 (6)	M	8-54	53.9	34.2	6.3	5.9	100	97	53	49	35	16	8.7	44	150+	2.2	2.404	5.2	72	18.6	100.0							
	Lane 2	M	8-54	57.3	31.3	6.2	5.2	100	99	50	40	31	17	6.5	46	150+	1.9	2.413	4.9	72	17.5	100.0						
		Lane 3	M	8-54	57.3	31.3	6.2	5.2	100	99	50	40	31	17	6.5	46	150+	1.9	2.413	4.9	72	17.5	100.0					
			Lane 4	M	8-54	55.5	33.4	6.0	5.1	100	97	51	42	33	15	6.3	43	150+	1.3	2.408	5.1	70	17.0	100.0				
				Lane 5 (6)	M	8-54	55.5	33.4	6.0	5.1	100	97	51	42	33	15	6.3	43	150+	1.3	2.408	5.1	70	17.0	100.0			
					Lane 6	M	8-54	57.8	30.2	6.8	5.4	100	98	51	39	30	16	7.2	46	150+	1.8	2.409	4.7	73	17.4	100.0		
						Lane 7	M	8-54	58.3	31.0	5.2	5.6	100	98	52	38	34	12	5.5	43	150+	0.9	2.413	4.5	74	17.3	100.0	
							Lane 8	M	8-54	55.5	33.7	5.6	5.2	100	98	52	41	32	15	6.3	44	150+	1.7	2.406	5.1	71	17.8	100.0

(a) Lanes numbered west to east.

(b) M = mix sample, C = core from center of lane, WT = core from wheel track. Dashes indicate cores taken at random locations.

(c) Penetrations of original asphalt =  $\begin{cases} 51 \text{ (Section 6B Lane 3, 4)} \\ 53 \text{ (Section 6B Lane 2)} \\ 54 \text{ (Section 6A all lanes)} \\ 55 \text{ (Section 6B Lane 1)} \end{cases}$

(d) Maximum theoretical specific densities =  $\begin{cases} 2.528 \text{ (Section 6B Lanes 2, 3)} \\ 2.535 \text{ (Section 6B Lanes 1, 4)} \\ 2.536 \text{ (Section 6A all lanes)} \end{cases}$

APPENDIX B

BINDER COURSE

Summary of Laboratory Analyses of Bituminous Concrete

- Section 1. Wyoming Crude, Refinery A
- Section 2. Venezuelan Crude
- Section 3. Wyoming Crude, Refinery B
- Section 4. West Texas Winkler Crude
- Section 5. Arkansas Smackover Crude
- Section 6. East Texas Talco Crude















APPENDIX C

Tabulation of Rutting in Wheel Tracks  
With Corresponding Vertical and Horizontal Alignments

- Section 1. Wyoming Crude, Refinery A
- Section 2. Venezuelan Crude
- Section 3. Wyoming Crude, Refinery B
- Section 4. West Texas Winkler Crude
- Section 5. Arkansas Smackover Crude
- Section 6. East Texas Talco Crude

RUT DEPTH MEASUREMENTS (inches)

Station	SBOL		SBIL		NBIL		NBOL		Plan Grade (NB)	Horizontal Alignment (NB)
	RWT	LWT	RWT	LWT	LWT	RWT	LWT	RWT		
SECTION 1. WYOMING CRUDE, REFINERY A										
SECTION 1A										
995	0.12	0.12	0.00	0.00	0.06	0.12	0.19	0.25	-2.57 % 600' VC	PC STRAIGHT SECTION
996	0.19	0.19	0.00	0.06	0.00	0.00	0.31	0.38		
997	0.19	0.12	0.00	0.12	0.06	0.00	0.12	0.19		
998	0.19	0.12	0.06	0.12	0.00	0.00	0.12	0.25		
999	0.19	0.09	0.06	0.12	0.00	0.12	0.31	0.19		
1000	0.31	0.31	0.06	0.12	0.00	0.00	0.06	0.19		
1001	0.12	0.12	0.06	0.06	0.00	0.00	0.12	0.12		
1002	0.19	0.06	0.00	0.03	0.00	0.00	0.06	0.12		
1003	0.12	0.06	0.00	0.00	0.00	0.00	0.25	0.19		
1004	0.09	0.00	0.00	0.06	0.00	0.00	0.19	0.12		
1005	0.06	0.06	0.06	0.06	0.00	0.00	0.06	0.12		
Average	0.16	0.11	0.03	0.07	0.01	0.02	0.16	0.19		
SECTION 1B										
1007	0.12	0.06	0.00	0.00	0.06	0.00	0.06	0.12	-2.20 % 400' VC	PT CURVE D=2° R
1008	0.12	0.00	0.00	0.00	0.06	0.06	0.25	0.25		
1009	0.06	0.00	0.03	0.00	0.00	0.00	0.12	0.12		
1010	0.00	0.00	0.06	0.03	0.06	0.00	0.28	0.12		
1011	0.00	0.00	0.00	0.00	0.00	0.00	0.25	0.12		
1012	0.00	0.00	0.00	0.00	0.00	0.00	0.25	0.19		
1013	0.06	0.12	0.06	0.06	0.00	0.00	0.00	0.19		
1014	0.12	0.12	0.00	0.03	0.06	0.00	0.12	0.12		
1015	0.16	0.12	0.00	0.19	0.00	0.00	0.19	0.25		
1016	0.12	0.06	0.00	0.06	0.06	0.06	0.25	0.25		
1017	0.12	0.12	0.06	0.12	0.00	0.00	0.50	0.31		
Average	0.08	0.05	0.02	0.04	0.03	0.01	0.21	0.19		
SECTION 2. VENEZUELAN CRUDE										
SECTION 2A										
1019	0.12	0.12	0.06	0.09	0.00	0.00	0.06	0.25	-0.50 % 500' VC	STRAIGHT SECTION
1020	0.12	0.16	0.06	0.12	0.06	0.00	0.03	0.19		
1021	0.16	0.12	0.06	0.09	0.00	0.00	0.12	0.19		
1022	0.12	0.12	0.06	0.06	0.06	0.06	0.06	0.25		
1023	0.12	0.31	0.03	0.06	0.00	0.06	0.19	0.19		
1024	0.12	0.12	0.06	0.12	0.00	0.06	0.06	0.31		
1025	0.12	0.12	0.06	0.06	0.06	0.06	0.19	0.31		
1026	0.09	0.12	0.06	0.19	0.06	0.00	0.19	0.38		
1027	0.19	0.12	0.06	0.09	0.00	0.00	0.06	0.25		
1028	0.19	0.19	0.06	0.12	0.06	0.03	0.25	0.38		
1029	0.19	0.12	0.00	0.03	0.00	0.06	0.19	0.38		
Average	0.14	0.15	0.05	0.09	0.03	0.03	0.13	0.28		
SECTION 2B										
1031	0.22	0.19	0.06	0.00	0.06	0.06	0.12	0.19	-0.10 % 400' VC	PC CURVE D=2° L
1032	0.19	0.16	0.09	0.06	0.06	0.00	0.06	0.22		
1033	0.12	0.06	0.06	0.00	0.06	0.06	0.06	0.25		
1034	0.12	0.12	0.03	0.06	0.06	0.00	0.16	0.19		
1035	0.16	0.12	0.00	0.06	0.00	0.00	0.09	0.22		
1036	0.19	0.16	0.00	0.00	0.06	0.06	0.06	0.12		
1037	0.31	0.19	0.16	0.00	0.06	0.06	0.25	0.38		
1038	0.12	0.12	0.12	0.06	0.00	0.06	0.12	0.25		
1039	0.12	0.19	0.06	0.00	0.06	0.00	0.12	0.19		
1040	0.12	0.19	0.06	0.06	0.06	0.00	0.12	0.19		
1041	0.12	0.25	0.06	0.03	0.06	0.03	0.06	0.19		
Average	0.16	0.16	0.06	0.03	0.05	0.03	0.11	0.22		

RUT DEPTH MEASUREMENTS (inches)

	Station	SBOL		SBIL		NBIL		NBOL		Plan Grade (NB)	Horizontal Alignment (NB)	
		RWT	LWT	RWT	LWT	LWT	RWT	LWT	RWT			
SECTION 3. WYOMING CRUDE, REFINERY A	SECTION 3A		1043	0.22	0.22	0.00	0.00	0.06	0.03	0.09	0.12	
	1044	0.19	0.25	0.00	0.06	0.00	0.06	0.00	0.19	0.19		
	1045	0.12	0.12	0.00	0.12	0.00	0.06	0.06	0.06	0.25		
	1046	0.22	0.22	0.00	0.00	0.00	0.12	0.00	0.12	0.19		
	1047	0.12	0.19	0.06	0.00	0.00	0.06	0.12	0.12	0.19		
	1048	0.38	0.19	0.00	0.00	0.12	0.06	0.12	0.12	0.19		
	1049	0.25	0.31	0.06	0.06	0.12	0.00	0.06	0.06	0.16		
	1050	0.16	0.16	0.00	0.00	0.06	0.12	0.06	0.06	0.25		
	1051	0.12	0.16	0.06	0.00	0.00	0.06	0.06	0.06	0.22		
	1052	0.19	0.19	0.12	0.12	0.06	0.09	0.12	0.12	0.12		
	1053	0.28	0.22	0.00	0.06	0.00	0.06	0.06	0.06	0.19		
Average	0.20	0.20	0.03	0.04	0.04	0.07	0.07	0.07	0.19			
SECTION 3B	1055	0.09	0.12	0.00	0.00	0.00	0.06	0.12	0.31	0.31		
	1056	0.19	0.12	0.00	0.09	0.06	0.00	0.12	0.34	0.34		
	1057	0.12	0.12	0.06	0.12	0.06	0.06	0.19	0.28	0.28		
	1058	0.25	0.25	0.00	0.09	0.06	0.06	0.12	0.25	0.25		
	1059	0.25	0.12	0.06	0.06	0.12	0.12	0.12	0.19	0.19		
	1060	0.19	0.06	0.00	0.12	0.06	0.06	0.06	0.12	0.12		
	1061	0.19	0.19	0.06	0.19	0.06	0.00	0.12	0.25	0.25		
	1062	0.25	0.19	0.06	0.06	0.06	0.06	0.06	0.25	0.25		
	1063	0.19	0.12	0.09	0.09	0.09	0.06	0.19	0.25	0.25		
	1064	0.34	0.19	0.12	0.00	0.06	0.06	0.31	0.44	0.44		
	1065	0.25	0.19	0.06	0.06	0.06	0.00	0.44	0.31	0.31		
Average	0.21	0.15	0.05	0.08	0.06	0.05	0.17	0.27	0.27			
SECTION 4. WEST TEXAS WINKLER CRUDE	SECTION 4A		1067	0.12	0.22	0.12	0.12	0.12	0.12	0.22	0.44	
	1068	0.12	0.25	0.09	0.12	0.00	0.00	0.38	0.44	0.44		
	1069	0.12	0.28	0.12	0.16	0.00	0.00	0.44	0.56	0.56		
	1070	0.12	0.25	0.12	0.12	0.12	0.00	0.19	0.31	0.31		
	1071	0.12	0.19	0.19	0.19	0.12	0.00	0.06	0.22	0.22		
	1072	0.12	0.19	0.00	0.06	0.06	0.00	0.19	0.31	0.31		
	1073	0.19	0.19	0.12	0.12	0.06	0.06	0.28	0.31	0.31		
	1074	0.09	0.19	0.06	0.06	0.00	0.00	0.38	0.38	0.38		
	1075	0.16	0.25	0.03	0.19	0.00	0.00	0.38	0.38	0.38		
	1076	0.12	0.12	0.06	0.12	0.00	0.38	0.44	0.38	0.38		
	1077	0.25	0.28	0.06	0.06	0.00	0.00	0.12	0.19	0.19		
Average	0.14	0.22	0.09	0.12	0.04	0.05	0.28	0.36	0.36			
SECTION 4B		1079	0.31	0.19	0.09	0.06	0.00	0.00	0.09	0.12		
1080	0.12	0.19	0.06	0.09	0.12	0.00	0.00	0.19	0.19			
1081	0.19	0.12	0.06	0.06	0.00	0.00	0.25	0.25	0.25			
1082	0.16	0.12	0.06	0.06	0.00	0.00	0.25	0.19	0.19			
1083	0.22	0.19	0.06	0.19	0.06	0.00	0.06	0.12	0.12			
1084	0.19	0.25	0.03	0.06	0.00	0.00	0.12	0.19	0.19			
1085	0.19	0.12	0.06	0.09	0.19	0.12	0.25	0.31	0.31			
1086	0.19	0.12	0.06	0.09	0.00	0.00	0.19	0.25	0.25			
1087	0.19	0.06	0.06	0.12	0.00	0.00	0.25	0.28	0.28			
1088	0.12	0.06	0.06	0.12	0.00	0.00	0.00	0.25	0.25			
1089	0.12	0.19	0.09	0.09	0.12	0.06	0.19	0.19	0.19			
Average	0.18	0.15	0.06	0.09	0.04	0.02	0.15	0.21	0.21			



RUT DEPTH MEASUREMENTS (inches)

Station	SBOL		SBIL		NBIL		NBOL		Plan Grade (NB)	Horizontal Alignment (NB)
	RWT	LWT	RWT	LWT	LWT	RWT	LWT	RWT		
SECTION 5. ARKANSAS SMACKOVER CRUDE										
SECTION 5A										
1091	0.09	0.12	0.00	0.00	0.00	0.00	0.25	0.25	+1.19 % 600' VC	PC STRAIGHT SECTION
1092	0.19	0.19	0.06	0.06	0.06	0.00	0.19	0.19		
1093	0.19	0.12	0.00	0.12	0.00	0.00	0.19	0.19		
1094	0.19	0.12	0.06	0.12	0.12	0.06	0.12	0.00		
1095	0.12	0.00	0.12	0.12	0.12	0.12	0.06	0.19		
1096	0.19	0.00	0.00	0.00	0.06	0.00	0.50	0.31		
1097	0.19	0.19	0.00	0.00	0.00	0.06	0.00	0.25		
1098	0.19	0.00	0.00	0.06	0.06	0.00	0.19	0.38		
1099	0.19	0.06	0.06	0.12	0.00	0.06	0.06	0.19		
1100	0.25	0.06	0.00	0.12	0.00	0.00	0.31	0.38		
1101	0.25	0.25	0.06	0.06	0.00	0.06	0.12	0.19		
Average	0.19	0.10	0.03	0.07	0.04	0.03	0.18	0.23		
SECTION 5B										
1103	0.12	0.12	0.00	0.12	0.06	0.12	0.44	0.28	+ 2.73 % 1300' VC	PT CURVE D=10 R
1104	0.19	0.06	0.12	0.06	0.06	0.12	0.19	0.25		
1105	0.19	0.12	0.12	0.19	0.12	0.12	0.31	0.25		
1106	0.00	0.12	0.06	0.00	0.06	0.06	0.19	0.19		
1107	0.31	0.06	0.06	0.12	0.00	0.12	0.50	0.25		
1108	0.19	0.12	0.06	0.12	0.06	0.06	0.19	0.25		
1109	0.06	0.09	0.06	0.12	0.06	0.00	0.75	0.25		
1110	0.19	0.25	0.06	0.09	0.06	0.06	0.50	0.25		
1111	0.19	0.12	0.03	0.12	0.06	0.00	0.31	0.19		
1112	0.12	0.12	0.06	0.00	0.00	0.00	0.38	0.25		
1113	0.12	0.12	0.06	0.00	0.00	0.00	0.38	0.25		
Average	0.15	0.12	0.06	0.09	0.05	0.06	0.38	0.24		
SECTION 6. EAST TEXAS TALCO CRUDE										
SECTION 6A										
1115	0.09	0.19	0.00	0.06	0.06	0.12	0.19	0.06	-1.56 % 500' VC	STRAIGHT SECTION
1116	0.09	0.06	0.06	0.09	0.00	0.00	0.12	0.25		
1117	0.12	0.12	0.12	0.16	0.12	0.06	0.12	0.25		
1118	0.09	0.12	0.06	0.06	0.00	0.00	0.19	0.25		
1119	0.12	0.09	0.00	0.12	0.12	0.06	0.19	0.00		
1120	0.12	0.09	0.03	0.06	0.06	0.00	0.12	0.06		
1121	0.19	0.12	0.06	0.12	0.00	0.00	0.25	0.25		
1122	0.16	0.06	0.09	0.12	0.12	0.00	0.19	0.25		
1123	0.06	0.19	0.06	0.12	0.00	0.00	0.12	0.19		
1124	0.19	0.12	0.06	0.06	0.00	0.06	0.12	0.19		
1125	0.16	0.06	0.06	0.00	0.06	0.00	0.19	0.12		
Average	0.13	0.11	0.05	0.09	0.05	0.03	0.16	0.17		
SECTION 6B										
1127	0.19	0.19	0.00	0.12	0.00	0.06	0.19	0.06	-1.03 % 600' VC	PT
1128	0.19	0.09	0.06	0.00	0.00	0.06	0.19	0.12		
1129	0.12	0.00	0.00	0.06	0.06	0.12	0.31	0.12		
1130	--	--	--	--	0.12	0.00	0.12	0.12		
1131	0.19	0.06	0.06	0.06	0.00	0.06	0.12	0.25		
1132	0.12	0.16	0.03	0.06	0.06	0.06	0.19	0.19		
1133	0.19	0.12	0.06	0.12	0.00	0.12	0.25	0.25		
1134	0.12	0.12	0.06	0.06	0.00	0.00	0.31	0.38		
1135	0.19	0.09	0.00	0.00	0.06	0.12	0.38	0.38		
1136	0.25	0.16	0.00	0.06	0.00	0.00	0.19	0.31		
1137	0.25	0.12	0.06	0.06	0.00	0.06	0.31	0.25		
Average	0.18	0.11	0.03	0.06	0.03	0.06	0.23	0.22		