## Michigan Department of Transportation Traffic and Safety

# STATEWIDE TRAFFIC SIGNAL DETAILS

## SIG-10-A THROUGH SIG-460-A

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### ENGLISH VERSION



PREPARED BY TRAFFIC AND SAFETY

DEPARTMENT DIRECTOR BRADLEY C. WIEFERICH, PE ENGINEER OF TRAFFIC AND SAFETY

DRAWN BY: MTS CHECKED BY: JAT

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Applicable	*One Completed Checklist required upon plan submittal for each contract package						
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NO.	ITEM
1	TRAFFIC SIGNAL (NO. HEADS AS INDICATED)
2	SPAN WIRE HANGER
3	5/16" EXTRA HIGH STRENGTH SPAN WIRE
4	3" SCHEDULE 80 PVC OR RIGID METAL (*) SEE NOTE ON SHEET 2 OF 5
5	P.J. TRAFFIC SIGNAL CABLE (AS SPECIFIED)
6	PREFORMED LASHING RODS
7	POLE BAND CLAMP (STEEL POLE)
8	GUY THIMBLE (2" DIA.)
9	(2) 5/8" X 4" LONG BOLT, NUT, WASHERS PER POLE BAND CLAMP (STEEL POLE)
10	INSTALL SPAN WIRE PREFORMED (ARMOR ROD) UNDER EACH SPAN HANGER
11	ACCESS HAND HOLE ALL NEW & EXISTING STEEL POLES
12	5/8" EYEBOLT, OVAL EYE AND NUT
13	WOOD MOULDING OR PLASTIC DUCT (10' FROM GROUND LEVEL)
14	3" SERVICE CAP (WEATHERHEAD), (METAL FOR STEEL AND PVC FOR WOOD POLES)
15	PREFORMED GUY GRIP DEAD ENDS

NOTES: (\*) FOR PROJECTS MAINTAINED BY THE WAYNE CO. DEPARTMENT OF PUBLIC SERVICES (WCDPS), USE RIGID METAL FOR CONDUIT(S) FROM GRADE LEVEL TO 10' (MÍN.) ABOVE GRADE OR AS DIRECTED BY THE ENGINEER.

(#) FOR PROJECTS MAINTAINED BY THE ROAD COMMISSION FOR OAKLAND COUNTY (RCOC), USE DRIP LOOP (WITHOUT COIL) FOR FUTURE ADJUSTMENT OF SIGNAL SPAN.

Michigan Department of Transportation	STANDARD PLAN FOR SPAN WIRE CASING DETAILS					
DEPARTMENT DIRECTOR	(SPECIAL DETAIL)	07/27/23		SHEET		
BRADLEY C. WIEFERICH, PE	FHWA APPROVAL	PLAN DATE	51G-010-A	2 OF 7		
				SECT		















#### DESCRIPTION:

Integral messenger wire consists of a support wire and a conductor core laid parallel and covered with a single extrusion of black low density polyethylene. The single extrusion provides a jacket over the support wire and core, and forms a web joining the two. See detail "A" this sheet. The support wire is 0.134 inches in diameter. Grade 190 steel, Class A galvanized, Extra High Strength steel having a rated breaking strength of 2680 pounds.

#### INSTALLATION:

Every effort shall be made to limit the length of spans to a maximum of 250 feet. Integral messenger wire is prone to low frequency wind vibration commonly referred to as "dancing" while "dancing" may not be so violent in low wind areas as to attract attention. Prolonged low amplitude vibration will eventually cause open circuits and/or support wire failure. Therefore, REA recommends that all integral messenger distribution wire be spiraled approximately one spiral for each 15 feet of span.

Spiraling of the wire should be done from every other pole by applying the spiraling torque to the support clamp after the two outside bolts have been properly tightened, thus keeping the spiraling torque on the support wire and not on the core. As spiraling operations proceed along a lead, spiraling at alternate poles should be in opposite directions, thereby reducing the torsion otherwise imposed on those clamps which are at the intermediate poles. The procedure to be followed in spiraling distribution wire is shown in detail "B" of this sheet.

If clamps are not adequately tightened the torsion developed in spiraling will cause the support wire to turn in the clamp resulting in the migration of the spirals from the spans toward the pole. "Dancing" of the wire and damage to it at the poles will be the final results of inadequate clamping. The proper type of support clamps must be used on all corners as shown in details "E", "F", "G" and "I" of this sheet.

When pulling the wire up to correct sag, a suitable wire grip should be used directly on the insulated support wire. The grip should be of such design as to give proper holding power and yet not damage the support wire jacket. The Crescent Tool Company #800 or an equivalent grip is suggested. A standard line wire grip should not be used because it will damage the insulation. If the insulation is damaged in any way, it must be repaired with sealing compound or by cutting out the damaged portion. At deadends, it is necessary to remove the support wire covering before applying the deadend grips. It must be done carefully to avoid damaging the support wire or core the eclectrical continuity of the support wire must be maintained throughout the lead.

MICHIGAN DEPARTMENT OF TRANSPORTATION	(SPECIAL DETAIL)			
BUREAU OF HIGHWAYS DELIVERY STANDARD PLAN	FHWA APPROVAL DATE		SIG = 011 = 1	SHEET
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#### STRAIN POLE DESIGN CRITERIA:

THE DESIGN OF THE STRAIN POLE STRUCTURES SHOWN ON SIG-021-A IS BASED ON THE AASHTO LRFD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES, AND TRAFFIC SIGNALS, FIRST EDITION (2015), WITH 2017 TO 2022 INTERIM REVISIONS.

THE DESIGN IS BASED ON THE MAXIMUM WIND AREAS AND WEIGHTS SHOWN BELOW IN THE SPAN WIRE LOADING TABLE. THE SUM OF THE WEIGHTS AND AREAS (VIEWED PERPENDICULAR TO THE SPAN) OF ALL SIGNALS, SIGNS, DEVICES AND OTHER EQUIPMENT SUPPORTED ON EACH SPAN MUST BE LESS THAN THE MAXIMUM VALUES SHOWN. THE SPAN LENGTH IS DEFINED AS THE STRAIGHT HORIZONTAL DISTANCE BETWEEN POLES. FOR TIE-OFF CONFIGURATIONS, THE SPAN LENGTH IS DEFINED AS THE LONGEST HORIZONTAL DISTANCE BETWEEN ANY TWO ADJACENT POLES MEASURED ALONG THE LENGTHS OF EACH WIRE.

#### LOAD PARAMETERS:

WIND LOAD (EXTREME EVENT LIMIT STATE): 1700-YEAR MRI BASIC WIND SPEED MAP, 120 MPH DESIGN WIND SPEED

WIND LOAD (SERVICE I EVENT LIMIT STATE): 10-YEAR MRI BASIC WIND SPEED MAP, 76 MPH DESIGN WIND SPEED

HEIGHT AND EXPOSURE FACTOR (K  $_{\rm Z}$ ): K  $_{\rm Z}$  IS CALCULATED USING A HEIGHT (Z) MEASURED FROM TOP OF THE DRILLED SHAFT (IT IS ASSUMED THAT THE POLE IS GROUND-MOUNTED).

DIRECTIONALITY FACTOR (K<sub>7</sub>): 0.85

GUST EFFECT FACTOR (G): 1.14

WIND DRAG COEFFICIENT FOR WIND AREA (Cd): 1.2

WIND LOAD REDUCTION FACTOR FOR FREE-SWINGING DEVICES SUPPORTED BY THE SPAN WIRE (UNTETHERED CONFIGURATIONS): 0.65

WIRE SAG: 5% (TYPICAL), 6.5% FOR SPANS WITH TIE-OFFS

STRAIN POLE LOADING: SEE SPAN WIRE LOADING TABLE BELOW. IN ADDITION, A DISAPPEARING LEGEND SIGN IS APPLIED MIDWAY ALONG ALL TIE-OFF SPANS (WHERE APPLICABLE). NO OTHER LOAD IS ALLOWED ON TIE-OFF SPANS.

SPAN WIRE + ELECTRIC CABLES WEIGHT: 1.155 LB/FT FOR 5/16" SPAN WIRE AND 1.467 LB/FT FOR 1/2" SPAN WIRE

#### SERVICABILITY PARAMETERS:

LIMIT FOR HORIZONTAL DEFLECTION AT TOP OF POLE UNDER DEAD LOAD ONLY: 2.5% OF POLE HEIGHT



NOTES:

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1. L1, L2 AND L3 ARE TIE-OFF SPANS.

2. SEE SIG-021 FOR POLE SIZES.

3. USE A SPAN LENGTH OF MAX(L1+L2, L2+L3) FOR DRILLED SHAFT LENGTH SELECTION FOR POLES CONNECTED TO TIE-OFF SPANS.

#### **DETAIL - STRAIN POLE CONFIGURATION**

SPAN WIRE LOADING TABLE							
SPAN LENGTH	WEIGHT OF GENERALIZED LOAD (LBS)**	MAX PERP. WIND AREA (TETHERED SPANS, SIGNALS WITH BACKPLATES) (SFT)**	MAX PERP. WIND AREA (UNTETHERED SPANS, SIGNALS WITHOUT BACKPLATES) (SFT)**				
UP TO 100 FT	328	58.0	31.0				
101 FT TO 120 FT	328	58.0	31.0				
121 FT TO 150 FT	328	58.0	31.0				
151 FT TO 175 FT	370	58.0	37.0				
176 ET TO 200 ET	370	58.0	37.0				

\*\*WEIGHTS AND AREAS INCLUDE ALLOWANCE FOR SIGNALS, SIGNS, AND ANYTHING ELSE CARRIED BY THE CABLES.



STRAIN POLE LOADING TABLE (POLE MOUNTED EQUIPMENT)						
EQUIPMENT	WEIGHT LBS	AREA SFT				
ANTENNA	20	0.09				
GPS MODULE	30	0.75				
POLE MOUNTED PEDESTRIAN SIGNAL	25	1.78				
1W-3C SIGNAL (WITH BACKPLATE)	43	8.67				
18' STEEL TRUSS BRACKET	106	7.29				
HEMISPHERICAL VIDEO DETECTION	22	2.11				
MICROWAVE VEHICLE DETECTOR	4	0.97				
RADAR VEHICLE DETECTOR	4	0.97				
DISAPPEARING LEGEND SIGN 45 9						

SEE ELEVATION VIEW FOR EQUIPMENT MOUNTING LOCATIONS.

APPROVED BY:	Michigan Department of Transportation	STANDARD PLAN FOR TRAFFIC SIGNAL STRAIN POLE LOADING TABLE AND DESIGN CRITERIA			
APPROVED BY:	DEPARTMENT DIRECTOR BRADLEY C. WIEFERICH, PE	(SPECIAL DETAIL) FHWA APPROVAL	07/27/23 PLAN DATE	SIG-020-A	SHEET 1 OF 1



		STRA	IN POLE	REQUIREN	<i>I</i> ENTS			
POLE LENGTH (FT)	Α	30 1	- T	36 FT	40 FT			
MIN. POLE WALL THICKNE	SS (IN)	W	0.429		0.625	0.625		
	AT TOP	Β′	8 (MIN) ± 1/2		8 (MIN) ± 1/2 8 (M		8 (MIN) ± 1/2	8 <sup>1</sup> /2 (MIN) ± 1/2
* POLE DIAMETER (IN)	AT BOTTOM	В	13 ± 1/2		13 ± 1/2	14 ± 1/2		
FULL LENGTH TAPER (IN/FT)			+0.002 0.14 -0.000		+0.002 0.14 -0.000	+0.002 0.14 -0.000		
POLE BASE FILLET WELD (IN)		E	5/16		1/2	1/2		
POLE BASE LANDING (IN)		F	3/8		5/8	5/8		
ANCHOR BOLT DIAMETER (IN)			11/2	1 3/4	1 <sup>3</sup> ⁄4	1 <sup>3</sup> /4		
ANCHOR BOLT HOLE DIAME	TER (IN)		1 <sup>13</sup> /16	21/16	2 <sup>1</sup> /16	2 <sup>1</sup> /16		
ANCHOR BOLT CIRCLE DIA	METER (IN)		18	21	21	21		
ANCHOR BOLT CHORD (IN)		С	9	10 <sup>1</sup> /2	10 <sup>1</sup> /2	10 <sup>1</sup> /2		
BASE PLATE EDGE (IN)		D	121/4	141/4	14 <sup>1</sup> /4	141/4		
BASE PLATE THICKNESS (IN)		T	2 21/4		21/4	21/4		
POLE BAND (SPAN CLAMP)		81/2"	P.O.C.H. 25'-6" TO 28'-6"		P.O.C.H. 32'-6" TO 34'-6"	P.O.C.H. 36'-0" TO 38'-6"		
		9' <sub>⁄2</sub> ″	P.O.C.H. 20'-6" TO 25'-0"		P.O.C.H. 25'-0" TO 32'-0"	P.O.C.H. 29'-0" TO 35'-6"		

\* DIAMETERS GIVEN ARE O.D.

NOTES:

- 1. THE DESIGN OF THIS STRUCTURE IS BASED ON THE AASHTO STANDARD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS, FIFTH EDITION, 2009.
- 2. S.S. DENOTES STAINLESS STEEL, GA. DENOTES GAUGE. O.D. DENOTES OUTSIDE DIAMETER. I.D. DENOTES INSIDE DIAMETER. H.S. DENOTES HIGH STRENGTH. SCH. DENOTES SCHEDULE.
- 3. P.O.C.H. IS THE POINT OF CONTACT HEIGHT OF THE POLE BAND CLAMP FROM THE BOTTOM OF THE STRAIN POLE.

POLE HEIGHT	
MFG	2 "
MO/YR OF MFG	
< 4″ >	

### STRAIN POLE STAINLESS STEEL ID TAG DETAIL

TO BE ATTACHED TO STRAIN POLE AT LOCATIONS SHOWN 4" FROM BASE OF TUBE BELOW HANDHOLE WITH (4) #8 X  $^3 \prime_8$  S.S. TYPE U DRIVE SCREWS. (LETTERS STAMPED IN 3/8" CHARACTERS)

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STRAIN POLE FOUNDATION REQUIREMENTS									
POLE LENGTH (FT)	30 F	T	36 FT	40 FT					
ANCHOR BOLT DIAMETER (IN)	11/2	1 <sup>3</sup> ′4	1 <sup>3</sup> ′4	1 <sup>3</sup> ′4					
ANCHOR BOLT CIRCLE DIAMETER (IN)	18	21	21	21					
ANCHOR BOLT LENGTH (IN)	72		72 72						
FOUNDATION DIAMETER (IN)	36	42	42	42					

NOTES:

- 1. ALL WORK AND MATERIALS MUST BE IN ACCORDANCE WITH THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION, CONSTRUCT STRAIN POLE FOUNDATIONS ACCORDING TO SECTION 718.03 OF THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION.
- 2. IF SOIL CONDITIONS INDICATE THERE IS NO NEED FOR A CASING PAY ITEM AS SHOWN ON THE PLANS. THE CONTRACTOR SHALL REQUEST PERMISSION OF THE ENGINEER TO INSTALL THE FOUNDATION WITHOUT CASING.
- 3. WHEN THE CASING PAY ITEM IS INCLUDING ON THE PLANS FOR A FOUNDATION (DUE TO GRANULAR SOILS OR A WET HOLE), STEEL CASING (SMOOTH WALLED) IS TO BE INSTALLED TO ENABLE THE FOUNDATION TO BE POURED. THE THICKNESS OF THE STEEL CASING IS TO BE DETERMINED BY THE CONTRACTOR. THE STEEL MUST BE LEFT IN PLACE. SMOOTH WALLED STEEL CASING OUTSIDE DIAMETER TO MEET OR EXCEED FOUNDATION DIAMETER. A SUITABLE METHOD OF COMPACTION MUST BE EMPLOYED TO ENSURE THE SOIL IMMEDIATELY OUTSIDE THE CASING IS COMPACTED PROPERLY.
- 4. WHEN THE CASING PAY ITEM IS CALLED FOR ON THE PLANS, THE STEEL CASING MAY STOP AT THE CONDUIT ENTRANCE TO FOUNDATION. TOP OF FOUNDATION MUST THEN BE FORMED SEPARATELY, EVEN THOUGH THE STEEL CASING STOPS AT THE CONDUIT ENTRANCE. THE CASING PAY ITEM QUANTITY WILL BE PAID FOR BASED ON ACTUAL LINEAR FEET INSTALLED.
- 5. DEWATERING OF WET SHAFTS IS NOT ALLOWED. A WET SHAFT IS DEFINED AS HAVING MORE THAN 3" OF STANDING WATER OR AS HAVING WATER INFILTRATING AT A RATE EQUAL TO OR EXCEEDING 12" PER HOUR. FOR WET SHAFTS, CONCRETE IS TO BE PLACED IN ACCORDANCE WITH SECTION 705 (WET CONSTRUCTION METHOD) WITH A TREMIE TUBE OR CONCRETE PUMP BEGINNING AT THE SHAFT BOTTOM. GRADE T CONCRETE MUST BE USED FOR UNDERWATER PLACEMENT. GRADE S2 MAY BE USED IN DRY EXCAVATIONS ONLY. SEE MDOT STANDARD SPECIFICATIONS TABLE 701-1 (CONCRETE STRUCTURE MIXTURES)
- 6. PER MDOT STANDARD SPECIFICATIONS 718.02, THE GRADE S2 ACCEPTABLE SLUMP RANGE IS 4-6 INCHES THE GRADE T ACCEPTABLE SLUMP RANGE IS 8-10 INCHES.
- 7. CONSTRUCT STRAIN POLE FOUNDATIONS, CASED OR UNCASED, ACCORDING TO SUBSECTION 810.03.J AND 705 OF THE STANDARD SPECIFICATIONS FOR CONSTRUCTION. ALL WORK AND MATERIALS MUST BE IN ACCORDANCE WITH THE MDOT STANDARD SPECIFICATION FOR CONSTRUCTION.
- 8. STEEL REINFORCEMENT MUST CONFORM TO SECTION 921 OF THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION.
- 9. EXPOSED CONCRETE SURFACES MUST BE CAST IN FORMS
- 10. STEEL REINFORCEMENT MUST HAVE A CLEAR COVER OF 3.00 INCHES UNLESS OTHERWISE NOTED. STEEL REIFORCEMENT MAY BE ADJUSTED TO ENSURE PROPER CLEAR COVER.
- 11. CONDUITS AND ANCHOR BOLTS MUST BE RIGIDLY INSTALLED BEFORE CONCRETE IS PLACED. ANCHOR BOLTS MUST BE SPACED BY MEANS OF A TEMPLATE. THE CENTER OF THE TEMPLATE MUST COINCIDE WITH THE CENTER OF THE FOUNDATION.
- 12. GROUNDING OF POLE INCLUDES ADDING #4 BARE COPPER GROUND WIRE BONDED BY LISTED MECHANICAL CONNECTION TO FOUNDATION REINFORCING STEEL AND HAVING 24" OF SLACK ABOVE THE TOP OF FOUNDATION.
- 13. INSTALL COPPER CLAD GROUND ROD(S) AS DIRECTED BY ENGINEER AND IN ACCORDANCE WITH CURRENT N.E.C. ALL GROUNDS MUST PROVIDE LESS THAN 10 OHM RESISTANCE TO GROUND.
- 14. REFER TO THE FOLLOWING SPECIAL PROVISIONS FOR 6 ANCHOR BOLT STRAIN POLES: STEEL STRAIN POLE STRAIN POLE FOUNDATION AND ANCHOR BOLTS CASING USED WITH STRAIN POLES AND MAST ARM POLES

#### NOT TO SCALE

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STRAIN POLE REQUIREMENTS									
				, I	NOT CON	NECTED TO TIE-OFI	CONNECTED TO TIE-OFF SPANS		
MARK	DESCRIPTION		UNIT	SIZ	E 1	SIZE 2	SIZE 3	SIZE 4	SIZE 5
A	POLE LENGTH		FT	30	0	36	40	36	40
W	MIN POLE WALL THICKNESS	GR 50 STEEL	IN	5/	8	5/8	5/8	5/8	5/8
		GR 65 STEEL	IN	1/:	2	1/2	1/2	1/2	1/2
B'	POLE DIAMETER - TOP *		IN	12.3 (MI	N) ± 1/2	13.96 (MIN) ± 1/2	14.4 (MIN) ± 1/2	14.96 (MIN) ± 1/2	15.4 (MIN) ± 1/2
В	POLE DIAMETER - BOTTOM *		IN	16.5 ±	± 1/2	19 ± 1/2	20 ± 1/2	20 ± 1/2	21 ± 1/2
	FULL LENGTH TAPER		IN / FT	+0.0	002	+0.002	+0.002	+0.002	+0.002
-				0.1	14	0.14	0.14	0.14	0.14
<u> </u>	1			-0.0	100	-0.000	-0.000	-0.000	-0.000
E	POLE BASE FILLET WELD	CR 50 STEEL		7/1	10	7/10	7/16	7/16	7/16
		GR 00 STELL		1/10		5/8	5/8	5/8	5/8
-			IN	4.34	2	2/0	3/0	3/0	3/0
<u> </u>				1 74		۷ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ ـ	۷	۷ د	4
-	ANCHOR BOLT HOLE DIAMET	ER	IN	2 <b>1</b> <sub>16</sub>	2 ¥ <sub>16</sub>	2 ¥ <sub>16</sub>	2 ¥ <sub>16</sub>	2 ¥ <sub>16</sub>	2 ¥ <sub>16</sub>
-	ANCHOR BOLT CIRCLE DIAME	ETER	IN	24	27	27	27	29	29
С	ANCHOR BOLT CHORD		IN	12	13 ½	13 1⁄2	13 <b>½</b>	14 1/2	14 1⁄2
D	BASE PLATE EDGE		IN	15 1⁄4	17 1/2	17 ½	17 1⁄2	20 <b>¼</b>	20 <b>¼</b>
Т	BASE PLATE THICKNESS		IN	2 <b>1</b> / <sub>4</sub>	2 1/4	2 1/4	2 <b>1</b> ⁄ <sub>4</sub>	2 <b>1</b> ⁄ <sub>4</sub>	2 <b>½</b>
POCH	12 1/2" POLE BAND CLAMP		FT-IN	21'-6" TC	D 28'-6"	-	-	-	-
P.O.C.H.	13 1/2" POLE BAND CLAMP		FT-IN	17'-0" TC	O 21'-0"	32'-6" TO 34'-6"	-	-	-
P.O.C.H.	14 1/2" POLE BAND CLAMP		FT-IN	-		25'-6" TO 32'-0"	32'-6" TO 38'-6"	32'-6" TO 34'-6"	-
P.O.C.H.	C.H. 15 ½" POLE BAND CLAMP		FT-IN	-	_	18'-0" TO 25'-0"	25'-6" TO 32'-0"	25'-6" TO 32'-0"	32'-6" TO 38'-6"
P.O.C.H.	.C.H. 16 ½" POLE BAND CLAMP		FT-IN	-		17'-0" TO 17'-6"	18'-0" TO 25'-0"	18'-0" TO 25'-0"	25'-6" TO 32'-0"
P.O.C.H.	17 ½" POLE BAND CLAMP		FT-IN	-		-	17'-0" TO 17'-6"	17'-0" TO 17'-6"	18'-0" TO 25'-0"
P.O.C.H.	18 ½" POLE BAND CLAMP		FT-IN	-		-	-	-	17'-0" TO 17'-6"
* DIAMET	ERS GIVEN ARE O.D.								

#### NOTES:

1. ROUND OR 12-SIDED SECTIONS ARE ALLOWED.

2. MULTI-PLY SECTIONS ARE NOT ALLOWED.

STRAIN POLES USED IN APPLICATIONS OTHER THAN TRAFFIC SIGNALS REQUIRE UNIQUE DESIGN CALCULATIONS FOR THE SPECIFIC LOADING CASE.
 E70 ELECTRODE REQUIRED FOR GR 50 STEEL. E80 ELECTRODE REQUIRED

FOR GR 65 STEEL.

#### ABBREVIATIONS:

S.S. = STAINLESS STEEL

GA. = GAUGE GR = GRADE

O.D. = OUTSIDE DIAMETER

I.D. = INSIDE DIAMETER

H.S. = HIGH STRENGTH

SCH = SCHEDULE

P.O.C.H. = POINT OF CONTACT HEIGHT OF THE POLE BAND CLAMP, MEASURED

FROM THE BOTTOM OF THE STRAIN POLE



SECT





STRAIN POLE FOUNDATION REQUIREMENTS										
NOT CONNECTED TO TIE-OFF SPANS CONNECTED TO TIE-OFF SPANS										
MARK	DESCRIPTION	UNIT	SIZE 1		SIZE 2	SIZE 3	SIZE 4	SIZE 5		
A	POLE LENGTH	FT	30	)	36	40	36	40		
	ANCHOR BOLT DIAMETER	IN	1 3⁄4	2	2	2	2	2		
	ANCHOR BOLT CIRCLE DIAMETER		24	27	27	27	29	29		
	ANCHOR BOLT LENGTH	IN	72		72	72	72	72		
	FOUNDATION DIAMETER IN		42	48	48	48	48	48		

### NOTES:

- ALL WORK AND MATERIALS MUST BE IN ACCORDANCE WITH THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION. CONSTRUCT STRAIN POLE FOUNDATIONS ACCORDING TO SECTION 718.03 OF THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION.
- 2. IF SOIL CONDITIONS INDICATE THERE IS NO NEED FOR A CASING PAY ITEM AS SHOWN ON THE PLAN, THE CONTRACTOR SHALL REQUEST PERMISSION OF THE ENGINEER TO INSTALL THE FOUNDATION WITHOUT CASING.
- 3. WHEN THE CASING PAY ITEM IS INCLUDING ON THE PLANS FOR A FOUNDATION (DUE TO GRANULAR SOILS, A WET HOLE OR OTHER UNSTABLE CONDITIONS), STEEL CASING (SMOOTH WALLED) IS TO BE INSTALLED TO ENABLE THE FOUNDATION TO BE POURED. THE THICKNESS OF THE STEEL CASING IS TO BE DETERMINED BY THE CONTRACTOR. THE STEEL MUST BE LEFT IN PLACE. SMOOTH WALLED STEEL CASING OUTSIDE DIAMETER TO MEET OR EXCEED FOUNDATION DIAMETER. IF SOIL EXTENDING LATERALLY WITHIN 6 FEET OF THE DRILLED SHAFT FOUNDATION IS LOOSENED OR OTHERWISE DISTURBED, SCARIFY MATERIAL AT THE BASE OF THE EXCAVATION BELOW THE DEPTH OF DISTURBANCE AND RECOMPACT IN ACCORDANCE WITH 206.03.B OF THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION. COMPACT ALL BACKFILL PLACED ABOVE THE RECOMPACTED BASE OF EXCAVATION IN ACCORDANCE WITH 206.03.B OF THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION.
- 4. WHEN THE CASING PAY ITEM IS CALLED FOR ON THE PLANS, THE STEEL CASING MAY STOP AT THE CONDUIT ENTRANCE TO FOUNDATION. TOP OF FOUNDATION MUST THEN BE FORMED SEPARATELY, EVEN THOUGH THE STEEL CASING STOPS AT THE CONDUIT ENTRANCE. THE CASING PAY ITEM QUANTITY WILL BE PAID FOR BASED ON ACTUAL LINEAR FEET INSTALLED.
- CONSTRUCT STRAIN POLE FOUNDATIONS, CASED OR UNCASED, ACCORDING TO SUBSECTION 820.03. AND 718 OF THE STANDARD SPECIFICATIONS FOR CONSTRUCTION. ALL WORK AND MATERIALS MUST BE IN ACCORDANCE WITH THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION.
- 6. STEEL REINFORCEMENT MUST CONFORM TO SECTION 905 OF THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION.
- 7. EXPOSED CONCRETE SURFACES MUST BE CAST IN FORMS.
- CONDUITS AND ANCHOR BOLTS MUST BE RIGIDLY INSTALLED BEFORE CONCRETE IS PLACED. ANCHOR BOLTS MUST BE SPACED BY MEANS OF A TEMPLATE. THE CENTER OF THE TEMPLATE MUST COINCIDE WITH THE CENTER OF THE FOUNDATION.
- 9. GROUNDING OF POLE INCLUDES ADDING #4 BARE COPPER GROUND WIRE BONDED BY LISTED MECHANICAL CONNECTION TO FOUNDATION REINFORCING STEEL AND HAVING 24" OF SLACK ABOVE THE TOP OF FOUNDATION.
- 10. INSTALL COPPER CLAD GROUND ROD(S) AS DIRECTED BY ENGINEER AND IN ACCORDANCE WITH CURRENT N.E.C. ALL GROUNDS MUST PROVIDE LESS THAN 10 OHM RESISTANCE TO GROUND.

	Michigan Department of Transportation		STA FOUNDATI	NDARD PLAN FOR ON REQUIREMENTS	
I	DEPARTMENT DIRECTOR	(SPECIAL DETAIL)	07/27/23		SHEET
I	BRADLEY C. WIEFERICH, PE	FHWA APPROVAL	PLAN DATE	310-021-A	5 OF 7





#### MAST ARM DESIGN CRITERIA:

THE DESIGN OF THE MAST ARM STRUCTURES SHOWN ON SIG-031-A, SIG-032-A & SIG-033-A IS BASED ON THE AASHTO LRFD SPECIFICATIONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES, AND TRAFFIC SIGNALS, FIRST EDITION (2015), WITH 2017 TO 2022 INTERIM REVISIONS.

LOAD PARAMETERS:

WIND LOAD (EXTREME EVENT LIMIT STATE): 1700-YEAR MRI BASIC WIND SPEED MAP, 120 MPH DESIGN WIND SPEED

WIND LOAD (SERVICE I EVENT LIMIT STATE): 10-YEAR MRI BASIC WIND SPEED MAP, 76 MPH DESIGN WIND SPEED

HEIGHT AND EXPOSURE FACTOR (K<sub>z</sub>): K<sub>z</sub> IS CALCULATED USING A HEIGHT (Z) MEASURED FROM TOP OF THE DRILLED SHAFT (IT IS ASSUMED THAT THE POLE IS GROUND-MOUNTED).

DIRECTIONALITY FACTOR (K<sub>d</sub>): 0.85

GUST EFFECT FACTOR (G): 1.14

MAST ARM LOADING: SEE MAST ARM LOADING TABLE BELOW. AREAS AND WEIGHTS SHOWN IN THE TABLE FOR SIGNALS INCLUDE BACKPLATES EXTENDING 5" FROM SIGNAL ON ALL SIDES. FOR TWIN MAST ARMS, THE LOADING IS THE SAME ON BOTH ARMS.

SERVICEABILITY PARAMETERS:

LIMIT FOR SLOPE AT TOP OF POLE UNDER DEAD LOAD ONLY: 0.35 IN/FT

LIMIT FOR MAST ARM VERTICAL DEFLECTION UNDER GALLOPING AND TRUCK GUST-INDUCED LOADING: 8 IN

FATIGUE PARAMETERS:

FATIGUE CATEGORY: SIG-031-A: CATEGORY I SIG-032-A: CATEGORY II SIG-033-A: CATEGORY III

MINIMUM BEND RADIUS FOR 16-SIDED SECTIONS: 5 X WALL THICKNESS OR 1", WHICHEVER IS GREATER

	MAST ARM LOADING TABLE																	
м	AST ARM I	DATA	TL1	(1W-3C S	BIGNAL)*	*	TL2 (1W-3C SIGNAL)		TL3 (1W-3C SIGNAL)			TL4 (1W-5C DOGHOUSE SIGNAL)						
SPAN FT	MTG HT SINGLE	MTG HT TWIN*	DISTANCE	WEIGHT LBS	AREA SFT	INC. 12"x27" SIGN?	DISTANCE	WEIGHT LBS	AREA SFT	INC. 12"x27" SIGN?	DISTANCE	WEIGHT LBS	AREA SFT	INC. 12"x27" SIGN?	DISTANCE	WEIGHT LBS	AREA SFT	INC. 12"x27" SIGN?
20 25 30		401.011.0					12'-0" 17'-0" 22'-0"	43	8.67	NO	20'-0" 25'-0" 30'-0"	43	8.67	NO	-		-	-
35 40 45	19'-0"	18'-6" & 21'-0"	-		-	_	19'-0" 24'-0" 29'-0"	75	10.92	YES	27'-0" 32'-0" 37'-0"	75	10.92	YES	35'-0" 40'-0" 45'-0"	69	13.72	NO
50	1		26'-0"**	75**	10.92**	YES**	34'-0"				42'-0"				50'-0"			

\*TWIN MAST ARMS NOT ALLOWED FOR CATEGORY III MAST ARMS. MINIMUM ALLOWABLE ANGLE BETWEEN ARMS IS 45 DEGREES. \*\*TL1 NOT ALLOWED FOR 50FT TWIN MAST ARMS

MAST ARM LOADING T		NT'D)					RE ANTENNA OR HEMISPHERICAL DETECTION	VIDEO
EQUIPMENT	WEIGHT LBS	AREA SFT		DETECTION OR			1	
ANTENNA	20	0.09		/	164			
GPS MODULE	30	0.75	ļ		TL3			
POLE MOUNTED PEDESTRIAN SIGNAL	25	1.78		ר [	т	2		
1W-3C SIGNAL (WITH BACKPLATE)	43	8.67	Т		<b> </b> ◄			
LUMINAIRE	50	1.00				TL1		
LUMINAIRE ARM	SEE SIG-031, S SIG-033 FOR	SIG-032, AND R DETAILS				]		D 1W-3C
HEMISPHERICAL VIDEO DETECTION	22	2.11		100 10 -	HH FT IR		SIGNAL (WITH	
MICROWAVE VEHICLE DETECTOR	4	0.97	•				BACKPLATES)	
RADAR VEHICLE DETECTOR	4	0.97						
24x96 STREET NAME SIGN	90	14.93		MICROWAVE VEHICLE		4'-6"		
				DETAIL - MA	POLE MOUNT PEDESTRIAN SIGNA		ON VIEW	
PROVED BY: DIRECTOR, BUREAU OF FIELD SEF	RVICES	Michigan Depa	IDOT rtment of Transportation	TRAFF	IC SIGNAL MA OADING TABL	NDARD PLAN FOR ST ARM POLE E AND DESIG	E AND MAST ARN N CRITERIA	Л
PROVED BY:	MENT	DEPART BRADLEY	MENT DIRECTOR C. WIEFERICH, PE	(SPECIAL DETAIL) FHWA APPROVAL	07/27/23 PLAN DATE	SIG	-030-A	SHEET 1 OF 1



	ROUND TAPERED STEEL MAST A	RM	
MAST ARM LENGTH	* MAST ARM DIMENSIONS	MTG HT SINGLE	MTG HT TWIN
20'-0"	0.1793"-16.50" × 13.70" × 20'-0"		
25'-0"	0.1793"-16.50" × 13.00" × 25'-0"	1	
30'-0"	0.1793"-16.50" x 12.30" x 30'-0"		
75/ 0//	0.4290"-16.50" × 13.70" × 20'-0"		
35 -0	0.1793"- ** x 12.00" x **		18'-6"
40/ 0//	0.4290"-16.50" × 13.70" × 20'-0"	19'-0"	8
40, -0,	0.1793"- ** x 11.30" x **		21'-0"
451 01	0.4290"-16.50" × 13.70" × 20'-0"	1	
45 -0	0.1793″- ** × 10.60″ × **		
50/ 0//	0.5000"-16.75" × 13.95" × 20'-0"		
50 -0	0.1793″- ** x 10.15″ x **		
MAST ARM T	UBE TAPER IS 0.140 IN/FT		

ROUND TAPERED STEEL MAST A	RM POLE
* POLE DIMENSIONS	LUMINAIRE ARM
0.4375"-18.00" × 14.78" × 23'-0"	NO
0.4375"-18.00" × 13.94" × 29'-0"	YES

POLE TUBE TAPER IS 0.140 IN/FT

\* DIAMETERS GIVEN ARE O.D.

\* DIAMETERS GIVEN ARE O.D.

\*\* TO BE DETERMINED BY CONTRACTOR BASED ON REQUIRED MAST ARM LENGTH AND TELESCOPIC SPLICE LENGTH.

#### NOTES:

- 1. THE DESIGN OF THIS STRUCTURE IS BASED ON THE 2001 AASHTO STANDARD SPECIFICATONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS FOR 90 MPH WIND LOAD AND CATEGORY I WITH GALLOPING, NATURAL WIND GUSTS, AND TRUCK INDUCED FATIGUE LOADS.
- 2. WELD THE LONGITUDINAL ARM SEAM ON THE INBOARD AND OUTBOARD SECTIONS OF THE TELESCOPIC FIELD SPLICE WITH A COMPLETE JOINT PENETRATION (CJP) WELD A MINIMUM OF 36 INCHES LONG. IN ADDITION, LONGITUDINAL SEAM WELDS MUST BE CJP FOR A MINIMUM OF 6 INCHES FROM TUBE TO PLATE CJP WELDS.
- 3. SEAM WELDS MUST BE 90° ± FROM HAND HOLE AT BASE.
- 4. LUMINAIRE ARM IS 11 GAUGE ROUND STEEL WITH 0.140 INCH PER FOOT TAPER.
- 5. BACKING BAR FOR PIPE TO BASE PLATE (P) AND MAST ARM TO MAST ARM PLATE MUST BE MINIMUM 5/16 INCH X 2 INCH PLATE.
- 6. 1/2 INCH DIAMETER (Ø) ROUND STOCK C-HOOK ATTACHED TO ALL POLE SIZES. 3/4 INCH SCHEDULE (SCH.) 40 PIPE ATTACHED TO ALL POLE SIZES AND INBOARD AND OUTBOARD ARM.
- 7. S.S. DENOTES STAINLESS STEEL. GA. DENOTES GAUGE. O.D. DENOTES OUTSIDE DIAMETER. I.D. DENOTES INSIDE DIAMETER. H.S. DENOTES HIGH STRENGTH.












TAPERED STEEL MAST ARM POLE					
POLE DIMENSIONS *	LUMINAIRE ARM	MAST ARM LENGTH (FT)			
0.4375"-18.00" x 14.78" x 23'-0"	NO	20.25.20.25.40.45			
0.4375"-18.00" x 13.94" x 29'-0"	YES	20, 25, 30, 35, 40, 45			
0.5000"-18.00" x 14.78" x 23'-0"	NO	50			
0.5000"-18.00" x 13.94" x 29'-0"	YES	50			

POLE TUBE TAPER IS 0.140 IN/FT \* DIAMETERS ARE GIVEN BY O.D.

TAPERED STEEL MAST ARM				
MAST ARM LENGTH	MAST ARM DIMENSIONS *	MTG HT SINGLE	MTG HT TWIN	
20'-0"	0.1793"-16.50" x 13.70" x 20'-0"			
25'-0"	0.1793"-16.50" x 13.00" x 25'-0"			
30'-0"	0.1793"-16.50" x 12.30" x 30'-0"			
35'-0"	0.4290"-16.50" x 13.70" x 20'-0" 0.1793"-14.30" x 12.00" x 16'-9"		18'-6"	
40'-0"	0.4290"-16.50" x 13.70" x 20'-0" 0.1793"-14.30" x 11.30" x 21'-9"	19'-0"	& 21'-6"	
45'-0"	0.4290"-16.50" x 13.70" x 20'-0" 0.1793"-14.30" x 10.60" x 26'-9"			
50'-0"	0.4290"-17.50" x 14.70" x 20'-0" 0.2391"-15.45" x 11.00" x 31'-11"			



**DETAIL - 16-SIDED SECTION** 

1

MAST ARM TUBE TAPER IS 0.140 IN/FT \* DIAMETERS ARE GIVEN BY O.D.



- 1. THE DESIGN OF THIS STRUCTURE IS BASED ON CATEGORY I GALLOPING, NATURAL WIND GUSTS, AND TRUCK INDUCED FATIGUE LOADS.
- 2. WELD THE LONGITUDINAL ARM SEAM ON THE INBOARD AND OUTBOARD SECTIONS OF THE TELESCOPIC FIELD SPLICE WITH A COMPLETE JOINT PENETRATION (CJP) WELD WITH A LENGTH EQUAL TO THE TELESCOPIC FIELD SPLICE LENGTH PLUS 6 INCHES. THE LAP OF THE ARM SECTIONS CANNOT EXTEND BEYOND THE LONGITUDINAL ARM SEAM CJP WELD WHEN THE FIELD SPLICE IS ERECTED AND IN ITS FINAL POSITION. IN ADDITION, LONGITUDINAL SEAM WELDS MUST HAVE 60% MINIMUM PENETRATION OR FUSION AND MUST BE CJP FOR A MINIMUM OF 6 INCHES FROM TUBE TO PLATE CJP WELDS.
- 3. SEAM WELDS IN MAST ARM POLE MUST BE 90° ± FROM HAND HOLE AT BASE. SEAM WELDS IN MAST ARM MUST BE LOCATED ON BOTTOM OF MAST ARMS.
- 4. LUMINAIRE ARM IS 11 GAUGE ROUND STEEL WITH 0.140 INCH PER FOOT TAPER.
- 5. BACKING BAR FOR PIPE TO BASE PLATE (  $\Box$  ) AND MAST ARM TO MAST ARM PLATE MUST BE MINIMUM 5/16 INCH X 2 INCH PLATE.
- 1/2 INCH DIAMETER (Ø) ROUND STOCK C-HOOK ATTACHED TO ALL POLE SIZES. 1 INCH SCHEDULE (SCH.) 80 PIPE ATTACHED TO ALL POLE SIZES AND INBOARD AND OUTBOARD ARM.
- S.S. DENOTES STAINLESS STEEL. GA. DENOTES GAUGE. O.D. DENOTES OUTSIDE DIAMETER. I.D. DENOTES INSIDE DIAMETER. H.S. DENOTES HIGH STRENGTH.
- 8. ROUND OR 16-SIDED SECTIONS ARE ALLOWED.
- 9. MULTI-PLY SECTIONS ARE NOT ALLOWED.
- 10. A VIBRATION MITIGATION DEVICE IS REQUIRED FOR 50 FOOT ARM AND SHALL BE INSTALLED AS CLOSE AS POSSIBLE TO THE END OF THE ARM.













	ROUND TAPERED STEEL MAST A	RM	
MAST ARM LENGTH	* MAST ARM DIMENSIONS	MTG HT SINGLE	MTG HT TWIN
20'-0"	0.2500"-8.50" x 5.70" x 20'-0"		
25'-0"	0.2500″-9.50″ × 6.00″ × 25′-0″		
30'-0"	0.2500″-10.50″ × 6.30″ × 30′-0″		
751 01	0.4290"-12.00" × 10.60" × 10'-0"		
35 -0	0.1793″- <del>**</del> x 7.50″ x <del>**</del>		18'-6"
10/ 0//	0.5000"-12.00" × 10.60" × 10'-0"	19'-0"	&
400.	0.1793″- ** × 6.80″ × **		21'-0"
15/ 0//	0.5000″-12.00″ × 9.90″ × 15′-0″		
45 -0 "	0.1793″- ** × 6.10″ × **		
50/ 0//	0.7500"-12.0" × 9.20" × 20'-0"		
50'-0"	0.1793″- ** x 5.36″ x **		

* POLE DIMENSIONS	LUMINAIRE ARM	MAST ARM LENGTH (FT)
0.313"-14.00" × 10.92" × 22'-0"	NO	20, 25
0.313"-14.00" × 9.94" × 29'-0"	YES	30, 35
0.358″-14.00″ × 10.92″ × 22′-0″	NO	40.45
0.358″-14.00″ × 9.94″ × 29′-0″	YES	101 15
0.478″-14.00″ × 10.92″ × 22′-0″	NO	50
0.478″-14.00″ × 9.94″ × 29′-0″	YES	50

ROUND TAPERED STEEL MAST ARM POLE

POLE TUBE TAPER IS 0.140 IN/FT

\* DIAMETERS GIVEN ARE O.D.

NOTE: ONLY USE THE MAST ARM LENGTHS WITH POLE SIZES AS INDICATED IN TABLE ABOVE

MAST ARM TUBE TAPER IS 0.140 IN/FT

\* DIAMETERS GIVEN ARE O.D.

\*\* TO BE DETERMINED BY CONTRACTOR BASED ON REQUIRED MAST ARM LENGTH AND TELESCOPIC SPLICE LENGTH.

#### NOTES:

- 1. THE DESIGN OF THIS STRUCTURE IS BASED ON THE 2001 AASHTO STANDARD SPECIFICATONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS FOR 90 MPH WIND LOAD AND CATEGORY II WITH GALLOPING, NATURAL WIND GUSTS, AND TRUCK INDUCED FATIGUE LOADS.
- 2. WELD THE LONGITUDINAL ARM SEAM ON THE INBOARD AND OUTBOARD SECTIONS OF THE TELESCOPIC FIELD SPLICE WITH A COMPLETE JOINT PENETRATION (CJP) WELD A MINIMUM OF 36 INCHES LONG. IN ADDITION, LONGITUDINAL SEAM WELDS MUST BE CJP FOR A MINIMUM OF 6 INCHES FROM TUBE TO PLATE CJP WELDS.
- 3. SEAM WELDS MUST BE 90° ± FROM HAND HOLE AT BASE.
- 4. LUMINAIRE ARM IS 11 GAUGE ROUND STEEL WITH 0.140 INCH PER FOOT TAPER.
- 5. BACKING BAR FOR PIPE TO BASE PLATE (P) AND MAST ARM TO MAST ARM PLATE MUST BE MINIMUM 5/16 INCH X 2 INCH PLATE.
- 6. 1/2 INCH DIAMETER (Ø) ROUND STOCK C-HOOK ATTACHED TO ALL POLE SIZES. 3/4 INCH SCHEDULE (SCH.) 40 PIPE ATTACHED TO ALL POLE SIZES AND INBOARD AND OUTBOARD ARM.
- 7. S.S. DENOTES STAINLESS STEEL. GA. DENOTES GAUGE. O.D. DENOTES OUTSIDE DIAMETER. I.D. DENOTES INSIDE DIAMETER. H.S. DENOTES HIGH STRENGTH.













TAPERED STEEL MAST ARM POLE						
POLE DIMENSIONS *	LUMINAIRE ARM	MAST ARM LENGTH (FT)				
0.313"-15.00" x 11.78" x 23'-0"	NO	20.25.20				
0.313"-15.00" x 10.94" x 29'-0"	YES	20, 25, 50				
0.429"-15.00" x 11.78" x 23'-0"	NO	25.9.40				
0.429"-15.00" x 10.94" x 29'-0"	YES	55 & 40				
0.500"-15.50" x 12.28" x 23'-0"	NO	45 9 50				
0.500"-15.50" x 11.44" x 29'-0"	YES	45 & 50				

POLE TUBE TAPER IS 0.140 IN/FT \* DIAMETERS ARE GIVEN BY O.D.

	TAPERED STEEL MAST ARM				
MAST ARM LENGTH	MAST ARM DIMENSIONS *	MTG HT SINGLE	MTG HT TWIN		
20'-0"	0.2500"-8.50" x 5.70" x 20'-0"				
25'-0"	0.2500"-9.50" x 6.00" x 25'-0"				
30'-0"	0.2500"-10.50" x 6.30" x 30'-0"				
35'-0"	0.4290"-13.50" x 12.10" x 10'-0" 0.1793"-12.68" x 8.96" x 26'-7"		18'-6"		
40'-0"	0.5000"-13.50" x 12.10" x 10'-0" 0.2391"-12.80" x 8.38" x 31'-7"	19'-0"	& 21'-0"		
45'-0"	0.4290"-15.00" x 12.90" x 15'-0" 0.2391"-13.61" x 9.18" x 31'-8"				
50'-0"	0.5000"-15.50" x 12.70" x 20'-0" 0.2391"-13.41" x 8.98" x 31'-8"				



MAST ARM TUBE TAPER IS 0.140 IN/FT \* DIAMETERS ARE GIVEN BY O.D.

#### NOTES:

- 1. THE DESIGN OF THIS STRUCTURE IS BASED ON CATEGORY II GALLOPING, NATURAL WIND GUSTS, AND TRUCK INDUCED FATIGUE LOADS.
- 2. WELD THE LONGITUDINAL ARM SEAM ON THE INBOARD AND OUTBOARD SECTIONS OF THE TELESCOPIC FIELD SPLICE WITH A COMPLETE JOINT PENETRATION (CJP) WELD WITH A LENGTH EQUAL TO THE TELESCOPIC FIELD SPLICE LENGTH PLUS 6 INCHES. THE LAP OF THE ARM SECTIONS CANNOT EXTEND BEYOND THE LONGITUDINAL ARM SEAM CJP WELD WHEN THE FIELD SPLICE IS ERECTED AND IN ITS FINAL POSITION. IN ADDITION, LONGITUDINAL SEAM WELDS MUST HAVE 60% MINIMUM PENETRATION OR FUSION AND MUST BE CJP FOR A MINIMUM OF 6 INCHES FROM TUBE TO PLATE CJP WELDS.
- 3. SEAM WELDS IN MAST ARM POLE MUST BE 90°  $\pm\,$  FROM HAND HOLE AT BASE. SEAM WELDS IN MAST ARM MUST BE LOCATED ON BOTTOM OF MAST ARMS.
- 4. LUMINAIRE ARM IS 11 GAUGE ROUND STEEL WITH 0.140 INCH PER FOOT TAPER.
- 5. BACKING BAR FOR PIPE TO BASE PLATE (  $\Box$  ) AND MAST ARM TO MAST ARM PLATE MUST BE MINIMUM 5/16 INCH X 2 INCH PLATE.
- 6. 1/2 INCH DIAMETER (Ø) ROUND STOCK C-HOOK ATTACHED TO ALL POLE SIZES. 1 INCH SCHEDULE (SCH.) 80 PIPE ATTACHED TO ALL POLE SIZES AND INBOARD AND OUTBOARD ARM.
- 7. S.S. DENOTES STAINLESS STEEL. GA. DENOTES GAUGE. O.D. DENOTES OUTSIDE DIAMETER. I.D. DENOTES INSIDE DIAMETER. H.S. DENOTES HIGH STRENGTH.
- 8. ROUND OR 16-SIDED SECTIONS ARE ALLOWED.
- 9. MULTI-PLY SECTIONS ARE NOT ALLOWED.



SECT









SECT



ROUN	D TAPERED STEEL MAST ARM
MAST ARM LENGTH	* MAST ARM DIMENSIONS
20'-0"	0.2500″-8.00″ × 5.20″ × 20′-0″
25′-0″	0.2500"-8.00" × 4.50" × 25'-0"
30′-0″	0.2500″-8.00″ × 3.80″ × 30′-0″
35′-0″	0.2500"-9.00" × 4.10" × 35'-0"
40′-0″	0.2500"-11.00" × 5.40" × 40'-0"
45′-0″	0.3125"-11.0" x 8.66" x 16'-9" 0.1793"- ** x 5.10" x **
50'-0"	0.3125"-11.0" × 8.66" × 16'-9" 0.1793"-9.30" × 4.40" × 35'-0"

ROUND TAPERED STEEL MAST AF	RM POLE
* POLE DIMENSIONS	ARM SPAN
0.3125"-14.00" × 11.06" × 21'-0"	6′-0″
0.3125"-14.00" × 9.80" × 30'-0"	8′-0″
0.3125"-14.00" × 8.96" × 36'-0"	10′-0″
0.3125"-14.00" × 8.40" × 40'-0"	24′-0″

POLE TUBE TAPER IS 0.140 IN/FT

\* DIAMETERS GIVEN ARE O.D.

MAST ARM TUBE TAPER IS 0.140 IN/FT

\* DIAMETERS GIVEN ARE O.D.

\*\* TO BE DETERMINED BY CONTRACTOR BASED ON REQUIRED MAST ARM LENGTH AND TELESCOPIC SPLICE LENGTH.

#### NOTES:

- 1. THE DESIGN OF THIS STRUCTURE IS BASED ON THE 2001 AASHTO STANDARD SPECIFICATONS FOR STRUCTURAL SUPPORTS FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS FOR 90 MPH WIND LOAD AND CATEGORY III WITH GALLOPING, NATURAL WIND GUSTS, AND TRUCK INDUCED FATIGUE LOADS.
- 2. WELD THE LONGITUDINAL ARM SEAM ON THE INBOARD AND OUTBOARD SECTIONS OF THE TELESCOPIC FIELD SPLICE WITH A COMPLETE JOINT PENETRATION (CJP) WELD A MINIMUM OF 36 INCHES LONG. IN ADDITION, LONGITUDINAL SEAM WELDS MUST BE CJP FOR A MINIMUM OF 6 INCHES FROM TUBE TO PLATE CJP WELDS.
- 3. SEAM WELDS MUST BE 90° ± FROM HAND HOLE AT BASE.
- 4. LUMINAIRE ARM IS 11 GAUGE ROUND STEEL WITH 0.140 INCH PER FOOT TAPER.
- 5. BACKING BAR FOR PIPE TO BASE PLATE (P) AND MAST ARM TO MAST ARM PLATE MUST BE MINIMUM 5/16 INCH X 2 INCH PLATE.
- 6. 1/2 INCH DIAMETER (Ø) ROUND STOCK C-HOOK ATTACHED TO ALL POLE SIZES. 3/4 INCH SCHEDULE (SCH.) 40 PIPE ATTACHED TO ALL POLE SIZES AND INBOARD AND OUTBOARD ARM.
- 7. S.S. DENOTES STAINLESS STEEL. GA. DENOTES GAUGE. O.D. DENOTES OUTSIDE DIAMETER. I.D. DENOTES INSIDE DIAMETER. H.S. DENOTES HIGH STRENGTH.

MFG DESIGN ARM LENGTH MO/YR OF MFG	MFG DESIGN POLE LENGTH MO/YR OF MFG	2 "	MDOT NO POLE TYPE	
ARM/POLE S.S. ID TO BE ATTACHED TO POLE OR MAS SHOWN 4" FROM BASE OF TUBE BELOW HA S.S. TYPE U DRIVE (LETTERS STAMPED IN 3/8"	TAG DETAIL T ARM AT LOCATIONS NDHOLE WITH (4) #8 X <sup>3</sup> /8" SCREWS. ' CHARACTERS)		BASE S.S. ID TAG DETAIL WELD TAG TO EDGE OF BASE @ (LETTERS STAMPED IN 3/8" CHARACTERS)	
MICHIGAN DEPARTMENT OF TRANSPORTATION				
BUREAU OF HIGHWAYS DELIVERY STANDARD PLAN	FHWA APPROVAL DATE		SIG-032-B	SHEET
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## TAPERED STEEL MAST ARM POLE

POLE DIMENSIONS*	LUMINAIRE ARM DIMENSIONS
0.4375"-14.00" x 11.06" x 21'-0"	6'-0"
0.4375"-14.00" x 9.80" x 30'-0"	8'-0"
0.4375"-14.00" x 8.96" x 36'-0"	10'-0"
0.4375"-14.00" x 8.40" x 40'-0"	24'-0"

POLE TUBE TAPER IS 0.140 IN/FT

\* DIAMETERS GIVEN ARE O.D.



#### NOTES:

- 1. THE DESIGN OF THIS STRUCTURE IS BASED ON CATEGORY III GALLOPING, NATURAL WIND GUSTS, AND TRUCK INDUCED FATIGUE LOADS.
- 2. WELD THE LONGITUDINAL ARM SEAM ON THE INBOARD AND OUTBOARD SECTIONS OF THE TELESCOPIC FIELD SPLICE WITH A COMPLETE JOINT PENETRATION (CJP) WELD WITH A LENGTH EQUAL TO THE TELESCOPIC FIELD SPLICE LENGTH PLUS 6 INCHES. THE LAP OF THE ARM SECTIONS CANNOT EXTEND BEYOND THE LONGITUDINAL ARM SEAM CJP WELD WHEN THE FIELD SPLICE IS ERECTED AND IN ITS FINAL POSITION. IN ADDITION, LONGITUDINAL SEAM WELDS MUST HAVE 60% MINIMUM PENETRATION OR FUSION AND MUST BE CJP FOR A MINIMUM OF 6 INCHES FROM TUBE TO PLATE CJP WELDS.
- 3. SEAM WELDS IN MAST ARM POLE MUST BE 90° ± FROM HAND HOLE AT BASE. SEAM WELDS IN MAST ARM MUST BE LOCATED ON BOTTOM OF MAST ARMS.
- 4. LUMINAIRE ARM IS 11 GAUGE ROUND STEEL WITH 0.140 INCH PER FOOT TAPER.
- 5. BACKING BAR FOR PIPE TO BASE PLATE (□) AND MAST ARM TO MAST ARM PLATE MUST BE MINIMUM 5/16 INCH X 2 INCH PLATE.
- 6. 1/2 INCH DIAMETER (Ø) ROUND STOCK C-HOOK ATTACHED TO ALL POLE SIZES. 1 INCH SCHEDULE (SCH.) 80 PIPE ATTACHED TO ALL POLE SIZES AND INBOARD AND OUTBOARD ARM.
- 7. S.S. DENOTES STAINLESS STEEL. GA. DENOTES GAUGE. O.D. DENOTES OUTSIDE DIAMETER. I.D. DENOTES INSIDE DIAMETER. H.S. DENOTES HIGH STRENGTH.
- 8. ROUND OR 16-SIDED SECTIONS ARE ALLOWED.
- 9. MULTI-PLY SECTIONS ARE NOT ALLOWED.



ATTACH TO POLE OR MAST ARM AT LOCATIONS SHOWN 4" FROM BASE OF TUBE BELOW HANDHOLE WITH (4) #8 x 3/8" S.S. TYPE U DRIVE SCREWS. (LETTERS STAMPED IN 3/6" CHARACTERS)

2 DETAIL - ARM/POLE S.S. ID TAG



WELD TAG TO EDGE OF BASE PLATE (LETTERS STAMPED IN 3/8" CHARACTERS)



Michigan	MDOT Department of Transportation	STANDARD PLAN FOR MAST ARM POLE AND MAST ARM REQUIREMENTS			
DEI BRAI	PARTMENT DIRECTOR DLEY C. WIEFERICH, PE	(SPECIAL DETAIL) FHWA APPROVAL	07/27/23 PLAN DATE	SIG-033-A	SHEET 2 OF 6
					SECT









### NOTES:

- 1) All ground rods shall be 3/4"x10' copper clad rod a minimum of 2 ground rods shall be used (one for the service disconnect and one for the messenger cable & pole).
- 2) Ground rod placement shall not be less than 12" from the foundation with a minimum of 6' between ground rods. Placement shall be as directed by the Engineer and in compliance with N.E.C.
- 3) Ground wire connection to grounding rod(s) shall utilize a non-solder type connection.
- 4) Indicate the direction of conduits in foundation top with an arrow.
- 5) [nstall pole that the foundation & anchor bolts are plumb.
- 6) All grounds shall provide less than 10 ohm resistance to ground.



NOT TO SCALE

**EMDOT** 

ENGINEER OF DEVELOPMENT (SPECIAL DETAIL) FHWA APPROVAL DATE

PLAN DATE

SIG-040-A

SHEET 1 of 4





NOT TO SCALE MICHIGAN DEPARTMENT OF TRANSPORTATION BUREAU OF HIGHWAYS DELIVERY STANDARD PLAN FHWA APPROVAL DATE

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# FHWA APPROVAL DATE SIG-040-A Rev. 02/16/17 PLAN DATE

SHEET 3 of 4 Foundation Notes:

- Refer to the following special provisions related to 6 anchor bolt mast arm poles: Traffic Signal Mast Arm Pole and Mast Arm Mast Arm Pole Foundation and Anchor Bolts Casing Used With Strain Poles and Mast Arm Poles
- 2. Templates shall be shop fabricated and assembled prior to being approved by MDDT for shipping.
- 3. Diameter of bolt holes in template shall be 1/16 " larger than anchor bolt diameter.
- 4. Conduits and anchor bolts shall be rigidly installed before concrete is placed. The center of the template shall coincide with the center of the foundation. The template and handles shall be well supported, horizontally level and firmly anchored in place a minimum of 24 hours after the concrete placement is completed.
- 5. Due care shall be taken during the concrete placement to avoid displacing the anchor bolts.
- 6. No hammering on the anchor bolts or template will be allowed.
- 7. After template is removed, thread nuts on to the bolt flush with the bolt end to protect threads until signal support is erected.
- 8. For anchor bolt material refer to section 908.14 A and B of the Michigan Standard Specifications for Construction. For anchor bolt installation and tightening refer to section 810.03 N.
- 9. Dewatering of wet shafts is not allowed. A wet shaft is defined as having more than 3 inches of standing water or as having water infiltrating at a rate equal to or exceeding 12 inches per hour. For wet shafts, Concrete is to be placed in accordance with section 718.03. (wet construction method) with a tremie tube or concrete pump beginning at the shaft bottom. Grade T concrete must be used for underwater placement. Grade S2 may be used in dry excavations only. See MDOT standard specifications Tables 701-1A and 701-1B (Concrete Structure Mixtures).
- 10. Per MDOT standard specifications 718.02, the Grade S2 acceptable slump range is 6-8 inches. The Grade T acceptable slump range is 7-9 inches.
- 11. If soil conditions indicate there is no need for a casing pay item as shown on the plans, the contractor should request permission of the engineer to install the foundation without casing.
- 12. When the casing pay item is included on the plans for a foundation (due to granular soils or a wet hole), steel casing (smooth walled) is to be installed to enable the foundation to be poured. The thickness of the steel is to be determined by the contractor. The steel casing shall be left in place. A suitable method of compaction must be employed to ensure the soil immediately outside the casing is compacted properly.
- 13. When the casing pay item is called for on the plans, the steel casing may stop at the conduit entrance to foundation. Top of foundation must then be formed separately. The casing pay item quantity will be paid for based on actual linear feet installed.
- 14. Construct mast arm foundations, according to subsections 718.03 of the Standard Specifications for Construction. All work and materials shall be in accordance with the MDDT Standard Specifications.
- 15. Steel reinforcement shall be ASTM A615 grade 60 without epoxy coating.
- 16. Exposed concrete surfaces shall be cast in forms. Exposed concrete edges shall be beveled 3/4".
- 17. Steel reinforcement shall have a clear cover of 3 inches unless noted otherwise. Steel Reinforcement may be adjusted to ensure proper clear cover.
- 18. Grounding of pole includes adding #4 bare copper ground wire bonded by mechanical connection to foundation reinforcing steel and having 24" of slack above the top of foundation.

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MICHIGAN DEPARTMENT OF TRANSPORTATION	(SPECIAL DETAIL)			
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## FOUNDATION NOTES:

- 1. TEMPLATES SHALL BE SHOP FABRICATED AND ASSEMBLED PRIOR TO BEING APPROVED BY MDOT FOR SHIPPING.
- 2. DIAMETER OF BOLT HOLES IN TEMPLATE SHALL BE 1/16" LARGER THAN ANCHOR BOLT DIAMETER.
- 3. MAST ARM ORIENTATION IS NOT DEPENDENT ON ANCHOR BOLT POSITION.
- 4. CONDUITS AND ANCHOR BOLTS SHALL BE RIGIDLY INSTALLED BEFORE CONCRETE IS PLACED. THE CENTER OF THE TEMPLATE SHALL COINCIDE WITH THE CENTER OF THE FOUNDATION. THE TEMPLATE AND HANDLES SHALL BE WELL SUPPORTED, HORIZONTALLY LEVEL, HELD VERTICAL AT THE PROPER ELEVATION, AND FIRMLY ANCHORED IN PLACE A MINIMUM OF 24 HOURS AFTER THE CONCRETE PLACEMENT IS COMPLETED.
- 5. DUE CARE SHALL BE TAKEN DURING THE CONCRETE PLACEMENT TO AVOID DISPLACING THE ANCHOR BOLTS.
- 6. NO HAMMERING ON THE ANCHOR BOLTS OR TEMPLATE WILL BE ALLOWED.
- 7. AFTER TEMPLATE IS REMOVED, THREAD NUTS ON TO THE BOLT FLUSH WITH THE BOLT END TO PROTECT THREADS UNTIL SIGNAL SUPPORT IS ERECTED.
- GALVANIZE ALL EXPOSED NUTS, BOLTS, AND WASHERS ACCORDING TO ASTM F2329. GALVANIZE ALL OTHER STEEL ITEMS ACCORDING TO ASTM A123.
  EMBEDDED NUTS, BOLTS, WASHERS, AND BOTTOM STEEL TEMPLATE NEED NOT BE GALVANIZED.
- 9. FOR ANCHOR BOLT MATERIAL, REFER TO SECTION 908.14 A AND B OF THE MICHIGAN STANDARD SPECIFICATIONS FOR CONSTRUCTION. FOR ANCHOR BOLT INSTALLATION AND TIGHTENING, REFER TO SECTION 810.03 N.
- 10. IF SOIL CONDITIONS INDICATE THERE IS NO NEED FOR A CASING PAY ITEM AS SHOWN ON THE PLANS, THE CONTRACTOR SHOULD REQUEST PERMISSION OF THE ENGINEER TO INSTALL THE FOUNDATION WITHOUT CASING.
- 11. WHEN THE CASING PAY ITEM IS INCLUDED ON THE PLANS FOR A FOUNDATION (DUE TO GRANULAR SOILS, A WET HOLE OR OTHER UNSTABLE CONDITIONS), STEEL CASING (SMOOTH WALLED) IS TO BE INSTALLED TO ENABLE THE FOUNDATION TO BE POURED. THE THICKNESS OF THE STEEL IS TO BE DETERMINED BY THE CONTRACTOR. THE STEEL CASING SHALL BE LEFT IN PLACE. IF SOIL EXTENDING LATERALLY WITHIN 6 FEET OF THE DRILLED SHAFT FOUNDATION IS LOOSENED OR OTHERWISE DISTURBED, SCARIFY MATERIAL AT THE BASE OF THE EXCAVATION BELOW THE DEPTH OF DISTURBANCE AND RECOMPACT IN ACCORDANCE WITH 206.03.B OF THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION. COMPACT ALL BACKFILL PLACED ABOVE THE RECOMPACTED BASE OF EXCAVATION IN ACCORDANCE WITH 206.03.B OF THE MDOT STANDARD SPECIFICATIONS FOR CONSTRUCTION.
- 12. WHEN THE CASING PAY ITEM IS CALLED FOR ON THE PLANS, THE STEEL CASING MAY STOP AT THE CONDUIT ENTRANCE TO FOUNDATION. TOP OF FOUNDATION MUST THEN BE FORMED SEPARATELY. THE CASING PAY ITEM QUANTITY WILL BE PAID FOR BASED ON ACTUAL LINEAR FEET INSTALLED.
- CONSTRUCT MAST ARM FOUNDATIONS, ACCORDING TO SUBSECTIONS 718.03 OF THE STANDARD SPECIFICATIONS FOR CONSTRUCTION. ALL WORK AND MATERIALS SHALL BE IN ACCORDANCE WITH THE MDOT STANDARD SPECIFICATIONS.
- 14. STEEL REINFORCEMENT SHALL BE ASTM A615 GRADE 60 WITHOUT EPOXY COATING.
- 15. EXPOSED CONCRETE SURFACES SHALL BE CAST IN FORMS. EXPOSED CONCRETE EDGES SHALL BE BEVELED 3/4".
- 16. GROUNDING OF POLE INCLUDES ADDING #4 BARE COPPER GROUND WIRE BONDED BY MECHANICAL CONNECTION TO FOUNDATION REINFORCING STEEL AND HAVING 24" OF SLACK ABOVE THE TOP OF FOUNDATION.

Michigan Department of Transportation	STANDARD PLAN FOR FOUNDATION NOTES			
DEPARTMENT DIRECTOR BRADLEY C. WIEFERICH, PE	(SPECIAL DETAIL)	07/27/23	SIG-040-B	SHEET
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CELL MODEM TRANSMITTER	
SEE "DETAIL B" FOR	
HOSE CLAMPS	
LMR 400DB TYPE CABLE	
1 1/2" DIAMETER BLACK PIPE (TYP) JAG SCREWS W/WASHERS (TYP)	
DRILL 9/16" HOLE IN ALUMINUM PLUG FOR CABLE	
IRUN CRUSS 12"	
SET SCREWS	
(*) NOTE: (PLASTIC OR METAL) RSU SHALL BE MOUNTED	
DETWEEN 19 AND 25	
<u>ANTENNA ATTACHMENT DETAIL (WOOD POLE)</u> FOR CELL MODEM TRANSMITTER AND RSU COMBO	
NOT TO SCALE	
MICHIGAN DEPARTMENT OF TRANSPORTATION (SPECIAL DETAIL) BUREAU OF HIGHWAYS DELIVERY STANDARD PLAN FHWA APPROVAL DATE 02/06/18 CIC-131 A	SHEET
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Red Red   Orange A   G Green   White - neutral G	
<u>STANDARD - 3 COLOR SIGNAL DISPLAY</u>	
Red SR = Steady   Orange SY = Steady Ye   White w/Black Stripe SY = Steady Ye   Green SG   White - neutral SG = Steady Gr	ellow Yellow een
<u>FLASHING YELLOW ARROW (FYA) - 4 COLOR SIGNAL DISPLAY</u>	
R Red R = Red Ball   Orange A "A, B, C, & D" phase   Green G G   White-black stripe TA   Blue GA   White - neutral GA	l Trow Tow
DOG HOUSE W/RIGHT TURNS - 5 COLOR SIGNAL DISPLAY	
COLOR CORE FOR WIRING CONNECTING TRAFFIC STONAL LANDS	
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#### NOTES:

- 1) The conduit shall be installed at a min. depth of 5'-6'' with reinforced bars (if required per R.R. requirements) as shown in detail and as indicated on the plans within the entire crossing of railroad R.O.W.
- 2) Track support devices shall be placed & removed to carry the railroad, tracks over the open trench installation with the trench properly supported with sheeting & any other supports required to complete the conduit installation within the railroad R.O.W. the extra cost of such installation shall be incidental to the encased conduit item.
- 3) Contractor shall contact Division Engineer of railroad before starting excavation for conduit installation.
- 4) Contractor shall reimburse railroad Co. for services performed by railroad Co. where required.
- 5) All work shall be conducted so as not to interfere with, interrupt or endanger the integrity of rail operation.





































	, 3 SECTION	<u>1</u> TYPE IV Re	2". 5 SECTION   Flective yellow tape border.		
	NOTES 1) Bc (f ir di 2) Bc ur 3) Dc 4) Us ar	: ickplates ixed supp istallatic rected by ickplates iless othe o not cut se one inc id visors. <u>TRAFFI(</u>	are to be used for mast arm type ort) or tethered span wire type ns as indicated on the plans or the Engineer. must be a one piece construction rwise directed by the Engineer. the backplate for installation. h border with yellow signal head	as	
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	ENGINEER OF DEVELOPMENT			SHEFT	
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## EXTENDER OPTION 8", 11", 19", 24", & 72" (CUT TO LENGTH)

#### NOTES

1.	BREAKAWAY LINK AND TURNBUCKLE ARE REQUIRED AT BOTH ENDS OF ALL BOTTOM TETHER SPANS, IF BREAKAWAY LINK BEGINS TO YIELD DURING INSTALLATION, IT SHALL BE REMOVED AND REPLACED, THE WIRE TENSION SHALL BE ADJUSTED TO MINIMIZE MOVEMENT OF SIGNAL HEADS IN HIGH WINDS, TYPICAL TENSION IS 500 TO 750 LBS.					
2.	INSTALL GROUND WIRE AT BOTH ENDS OF TOP AND BOTTOM TETHERING.					
3.	IF SIGNAL ORIENTATION IS NOT PERPENDICULAR TO SPAN AND TETHER WIRE, THEN USE AN ANCHOR EXTENSION. CLAMP ASSEMBLY MUST BE ATTACHED TO THE FLAT SIDE OF THE EXTENDER BAR.					
4.	BREAKAWAY LINK SHALL YIELD AT 3325 LBS OF	TENSION.				
5.	TETHER WIRE SHALL BE 7-STRAND ASTM A475 HS 1/4 INCH. ON ALL SPANS, INSTALL TETHER HORIZONTALLY. MAINTAIN CLEARANCE OF 17' OVER ROADWAY.					
6.	GROUNDING WIRE ANCHOR HEIGHT TO THE SPAN WIRE IS ADJUSTED IN THE FIELD BEFORE BREAKAWAY LINK IS INSTALLED. GROUNDING WIRE LENGTH SHALL BE ADJUSTED SO THAT THE MINIMUM VERTICAL CLEARANCE OF THE SAGGING TETHER WIRE ABOVE THE PAVEMENT WITHOUT THE BREAKAWAY LINK INSTALLED IS AT LEAST 14'. GROUNDING WIRE SHALL CONTAIN ENOUGH SLACK FOR HEAD TO SWAY IN HIGH WINDS. GROUNDING WIRE SHALL BE ATTACHED TO THE SPAN WIRE USING A TINNED COPPER SPLIT BOLT.					
<ul> <li>TRAFFIC SIGNAL HOUSING REINFORCEMENT PLATES ARE REQUIRED WHEN TETHERING.</li> <li>A. TOP TETHERING REQUIRES REINFORCEMENT PLATES AT THE TOP OF THE POLYCARBONATE HOUSING. (2 PLATES TOTAL)</li> <li>B. BOTTOM TETHERING REQUIRES REINFORCEMENT PLATES AT THE TOP AND BOTTOM OF POLYCARBONATE HOUSING. (4 PLATES TOTAL)</li> <li>C. BOTTOM TETHERING 5-SECTION HEADS REQUIRES REINFORCEMENT PLATES ON THE TOP AND BOTTOM OF THE POLYCARBONATE HOUSING (8 TOTAL) AND AN ALUMINUM TRI-STUD UPPER ARM ASSEMBLY IN PLACE OF THE BOTTOM BRACKET.</li> <li>D. IF POLYCARBONATE HOUSING CONNECTS TO AN ALUMINUM CASE SIGN, REINFORCEMENT PLATES MUST BE USED AT THE CONNECTION.</li> </ul>						
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#6 COPPER MIN GROUNDING WIRE FROM OVERHEAD WEATHERHEAD

SPAN WIRE CLAMP



#### NOTES:

- 1. BREAKAWAY LINK AND TURNBUCKLE ARE REQUIRED AT BOTH ENDS OF ALL BOTTOM TETHER SPANS. IF BREAKAWAY LINK BEGINS TO YIELD DURING INSTALLATION, IT SHALL BE REMOVED AND REPLACED. THE WIRE TENSION SHALL BE ADJUSTED TO MINIMIZE MOVEMENT OF SIGNAL HEADS IN HIGH WINDS. TYPICAL TENSION IS 500 TO 750 LBS.
- 2. INSTALL GROUND WIRE AT BOTH ENDS OF TOP AND BOTTOM TETHERING.
- 3. IF SIGNAL ORIENTATION IS NOT PERPENDICULAR TO SPAN AND TETHER WIRE, THEN USE AN ANCHOR EXTENSION. CLAMP ASSEMBLY MUST BE ATTACHED TO THE FLAT SIDE OF THE EXTENDER BAR.
- 4. GROUNDING WIRE ANCHOR HEIGHT TO THE SPAN WIRE IS ADJUSTED IN THE FIELD BEFORE BREAKAWAY LINK IS INSTALLED. GROUNDING WIRE LENGTH SHALL BE ADJUSTED SO THAT THE MINIMUM VERTICAL CLEARANCE OF THE SAGGING TETHER WIRE ABOVE THE PAVEMENT WITHOUT THE BREAKAWAY LINK INSTALLED IS AT LEAST 14', GROUNDING WIRE SHALL CONTAIN ENOUGH SLACK FOR HEAD TO SWAY IN HIGH WINDS. GROUNDING WIRE SHALL BE ATTACHED TO THE SPAN WIRE USING A TINNED COPPER SPLIT BOLT.
- 5. TRAFFIC SIGNAL HOUSING REINFORCEMENT PLATES ARE REQUIRED WHEN TETHERING.
  - A. TOP TETHERING REQUIRES REINFORCEMENT PLATES AT THE TOP OF THE POLYCARBONATE HOUSING. (2 PLATES TOTAL)
     B. BOTTOM TETHERING REQUIRES REINFORCEMENT PLATES AT THE TOP AND
  - B. BOTTOM TETHERING REQUIRES REINFORCEMENT PLATES AT THE TOP AND BOTTOM OF POLYCARBONATE HOUSING. (4 PLATES TOTAL)
     C. BOTTOM TETHERING 5-SECTION HEADS REQUIRES REINFORCEMENT PLATES
  - ON THE TOP AND BOTTOM OF THE POLYCARBONATE HOUSING (12 TOTAL) AND AN ALUMINUM TRI-STUD UPPER ARM ASSEMBLY IN PLACE OF THE BOTTOM BRACKET.
  - D. IF POLYCARBONATE HOUSING CONNECTS TO AN ALUMINUM CASE SIGN, REINFORCEMENT PLATES MUST BE USED AT THE CONNECTION.



### DETAIL - OVERHEAD LANE ASSIGNMENT SIGN CONNECTION

#### NOTES:

- 1. ALUMINUM ANGLES SHALL BE USED FOR ALUMINUM SIGNS (TYPE III).
- 2. STEEL OR ALUMINUM ANGLES MAY BE USED ON PLYWOOD SIGNS (TYPE II).

# DEPARTMENT DIRECTOR Image: Standard Plan For DEPARTMENT DIRECTOR EXTENDER OPTION AND OVERHEAD LANE BRADLEY C, WIEFERICH, PE (SPECIAL DETAIL) OT/27/23 SIG-305-D FHWA APPROVAL PLAN DATE
















































	Aluminum cap (Typ) Iron cross (Typ) Aluminum cap (Typ) Iron cross Iron cross SIGNAL MOUNTING HARDWARE F	C stance 1/2"	DE BRACKET	
ΝΩΤ ΤΩ SCALE	SIGNAL MOUNTING HARDWARE -	Top bracket assembly Post top Pedestal Bac as STANDARD	) pttom bracket sembly <u>BRACKET</u> Web/Sp Det/Fin/S10331A.dan Rev.	
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