

MICHIGAN
STATE HIGHWAY DEPARTMENT
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NON-SKID BITUMINOUS SURFACE TREATMENTS

PROJECT M5-17, C2 AND M89-5, C3

By

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NON-SKID BITUMINOUS RESURFACE TREATMENT ON
PROJECTS M5-17, C2 AND M89-5, C3

In June, 1940 the Department constructed a non-skid bituminous surface on project M5-17, C2 in Antrim County using asphaltic materials MC-1B for the prime course and a blended native lake asphalt (LNA) for the surface course. At the same time project M89-5, C3 in adjoining Otsego County was constructed using tar T-3 for the prime course and tar T-9 for the surface course. Both projects prior to receiving the surface treatments had normal gravel road surfaces. The surface treatments were applied to both projects under similar working conditions by the same contractor, A. E. Hodgkiss, of Petoskey, Michigan.

Project M5-17, C2 is located on route US 131 between Hancock and Klairia and project M89-5, C3 extends from Klairia to Gaylord on route M 32. See Figure 1. The type of surface treatment which each project received is described below in Table I. Specifications for the materials used in the work are given in Tables II A, B, C and D at the end of the report.

Table I

Comparison of Surface Treatments

	<u>Project M5-17, C2</u>	<u>Project M89-5, C3</u>
Prime Coat and Cover	MC-1B at 0.2 gal./sq. yd. 26B aggregate, 10#/sq. yd.	T-3 at 0.2 gal./sq. yd. 26B aggregate, 10#/sq. yd.
1st Surface Course	LNA at 0.2 gal./sq. yd. 26B aggregate, 20#/sq. yd.	T-9 at 0.2 gal./sq. yd. 26B aggregate, 20#/sq. yd.
2nd Surface Course	LNA at 0.2 gal./sq. yd. 22A aggregate, 20#/sq. yd.	T-9 at 0.2 gal./sq. yd. 22A aggregate, 20#/sq. yd.

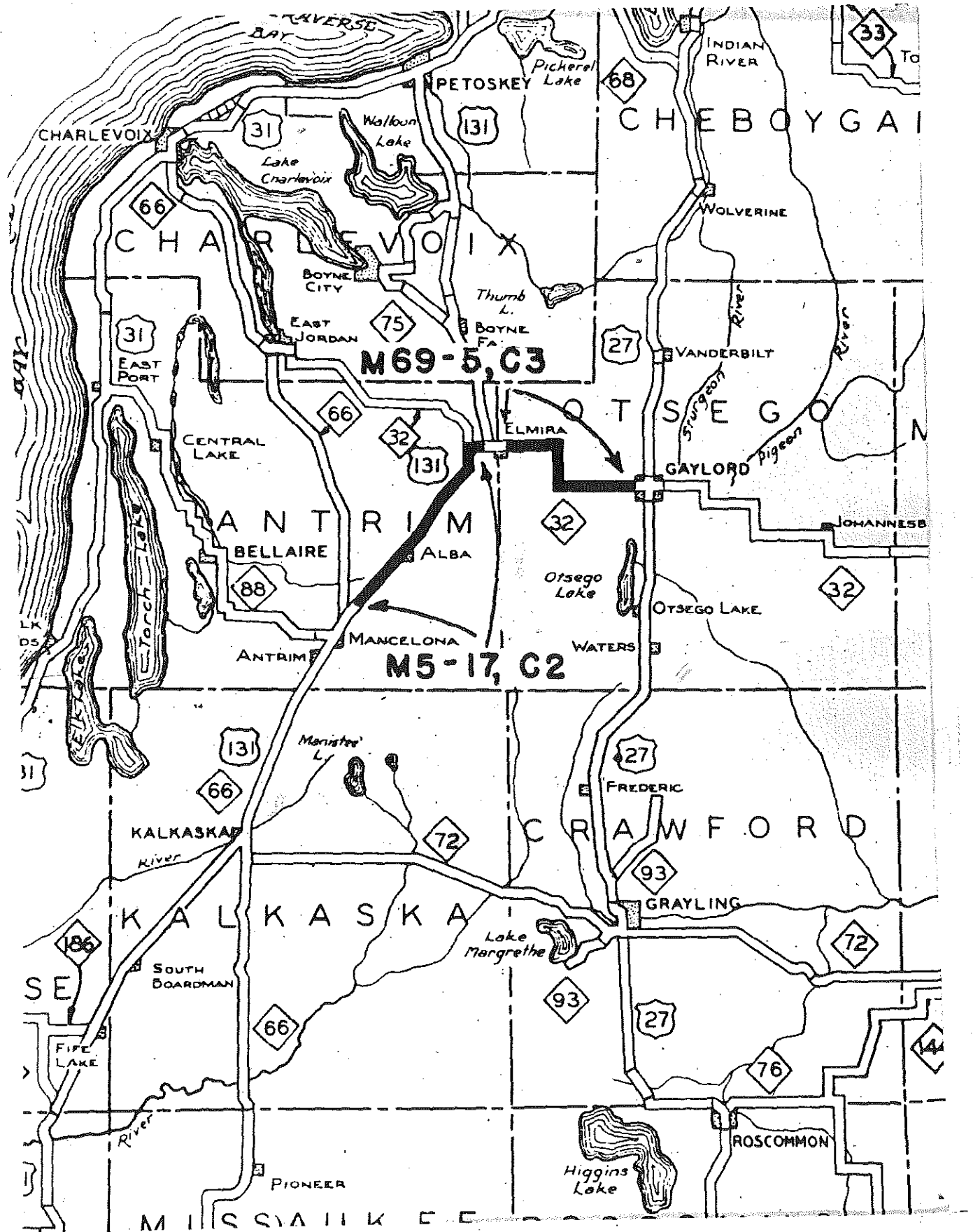


Figure 1.

A visual survey was made of both projects on September 5, 1948 for the purpose of evaluating the relative service behavior of these two types of surface treatments. Unfortunately for the purpose of our study, the two projects had been given a single seal coat treatment in 1945 consisting of tar and gravel. Consequently, no conclusions could be drawn from the relative appearance of the two surfaces, although both projects were in excellent condition due to the recent surface treatment. See Figures 2 and 3. In applying the single seal coat, tar T-5 was used on project MS-17,02 over the asphalt surface treatment and T-9 was used on MS-5,03. The gravel material in both cases was M1B. In spite of the fact that both projects appeared in excellent condition, it was quite evident that the tar project MS-5,03 had received extensive edge patching throughout its entire length which was not true in the case of the asphalt project MS-17,02. This fact is clearly brought out in Figures 2 and 3. Traffic records indicate that project MS-5,03 receives slightly more traffic than MS-17,02. For example, 1941 traffic records give an average 24 hour daily traffic flow of 747 vehicles on MS-5,03 as compared to 660 vehicles on MS-17,02.

Since it was practically impossible to evaluate the service performance of the two projects by visual inspection, because of the recent surface treatment, a study was made of maintenance cost records for the period 1940 to 1945. A summary of maintenance costs for these years are presented in Table II. The cost figures in Table II show that the average yearly maintenance costs on these two projects are materially different. In fact the average yearly maintenance cost for the asphalt project MS-17,02, as shown in Table II, is approximately 1/3 that of project MS-5,03.



GENERAL VIEW 

CONDITION OF SURFACE
PROJECT 5-17, C2.....1946
(FIGURE 2.)

TEXTURE of SURFACE 





GENERAL VIEW



CONDITION OF SURFACE
PROJECT M69-5, C3...1946
(AFTER 2ND SURFACE TREATMENT FIGURE 3)



TEXTURE of SURFACE

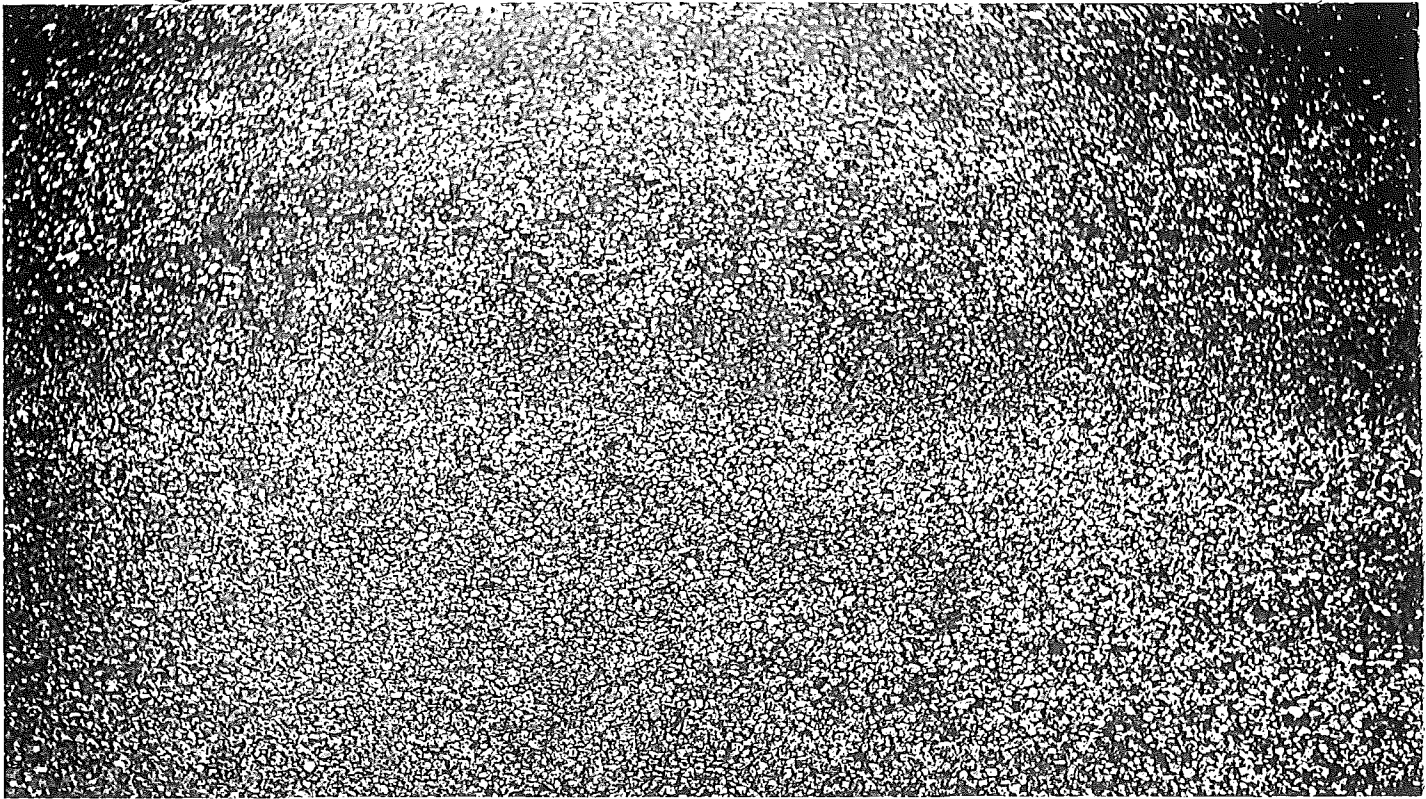


TABLE II

SUMMARY OF MAINTENANCE COSTS, PROJECTS MS-17, CR AND MS9-5, CS

Year	Routine Surface Patching	Bituminous Surface Treatment	Total Cost	Average Cost Per Mile
Project	MS-17, CR	Length 12.5 M	\$20,085.95	\$1,606.87
1940	588.43	189.72	778.15	62.25
1941	836.89	-	836.89	66.95
1942	619.04	278.85	897.89	71.83
1943	468.28	*12,834.11	468.28	37.80
1944	887.90	84.00	971.90	78.95
1945	808.81	-	808.81	65.71

Total Average Maintenance Cost Per Mile \$90.83

Project	MS9-5, CS	Length 10.0 M	\$15,375.24	\$1,537.52
1940	1,081.44	23.81	1,084.75	108.48
1941	3,550.74	23.81	3,574.05	357.41
1942	2,055.71	-	2,055.71	205.57
1943	1,548.58	*12,801.42	1,548.58	154.86
1944	519.93	-	519.93	51.99
1945	2,350.04	-	2,350.04	235.00

Total Average Maintenance Cost Per Mile \$185.55

* Total cost of single seal course which was laid in 1943 is not included in average yearly maintenance costs.

The information presented above would indicate that the surface treatment containing native lake asphalt for the binder material was giving better performance than the surface treatment constructed with tar products, a fact which should not be overlooked by the Department.

TABLE II-1
 SPECIFICATIONS FOR CUT BACK ASPHALT MC-1B
 1940

The material shall be homogeneous and free from water, shall not foam when heated to the required temperature for proper application, and shall conform to the requirements given below.

Flash Point (Taylor's Open Cup) degrees C	28.74
Viscosity, Saybolt Furol, sec.	40 - 150
Distillation, percent by volume	
To 100°C.	0
To 225°C.	10+
To 315.5°C.	25+
To 380°C.	45-
Distillation Residue	
Penetration at 25°C., 100 g., 5 sec.	100 - 300
Ductility at 25°C., cm.	60+
Solubility in C Cl ₄ , percent	99.5+
Olefinic Spot Test	Negative

TABLE II-B
SPECIFICATIONS FOR LIQUID ASPHALT, LMA

1940

The liquid asphalt shall be a blended native lake asphalt. It shall be homogeneous, free from water and shall not foam when heated to the required temperature for proper application. It shall conform to the following requirements:

	Min.	Max.
Specific Gravity, 15.5°/15.5°C	1.02	
Flash Point (Cleveland Open Cup) degrees C	120.5	
Float Test at 50°C., Sac.	100	150
Total Bitumen (Soluble in CS ₂) per cent	90	
Non-bituminous material naturally occurring therein (Insoluble in CS ₂) per cent	3	
Loss on Heating at 165°C., 50g., 5 hrs., per cent		2
Asphalt Content of 100 penetration when evaporated at 232.2°-260°C. per cent	80	

Car heating equipment will be required to maintain a heat of 300-350°F. to facilitate unloading tank cars and to load the distributor. A pressure distributor will be required which is equipped to maintain a temperature of 300-325°F. for application.

TABLE II-C
 SPECIFICATIONS FOR TARS T-3, T-8 AND T-9

All tars shall be homogeneous and shall be a product consisting entirely of materials derived from the high temperature carbonization of coal, refined to the specified consistency as necessary to conform to the requirements of the type or grade specified for the work.

Michigan Designation	T-3	T-9	T-8
Year	(1940)	(1940)	(1942)
Water, percent by volume	2.0-	0	0
Specific Gravity, 25°/25°C.	1.00+	1.14+	1.14+
Specific Viscosity Engler at 40°C.	15-22	-	-
Float test at 52°C., sec.	-	120-200	80-120
Total Bitumen Percent by weight	88+	78+	78+
Distillation, Test on Water Free Material, percent by weight			
To 170°C.	7.0-	1.0-	1.0-
To 270°C.	50.0-	15.0-	15.0-
To 300°C.	40.0-	25.0-	25.0-
Softening Point, R&B, of Distillation Residue, degree C.	35-40	35-55	35-55

TABLE II-D

SPECIFICATIONS FOR COARSE AGGREGATE COVER MATERIAL

The coarse aggregates shall conform to the material grading and physical requirements given in the following table.

Michigan Designation	26B	32A	31B
Year	(1940)	(1940)	(1942)
Material	Gravel	Gravel	Gravel

Grading Requirements

Passing 1 inch sieve %			
Passing 3/4 inch sieve %			
Passing 1/2 inch sieve %	100		
Passing 3/8 inch sieve %	65-85	100	100
Passing No. 4 inch sieve %	15-45	55-75	-
Passing No. 10 inch sieve %	0-10	0-15	0-15

Physical Requirements

Deval Abrasion			
Uncrushed, % max.	15	15	15
Crushed, % max.	25	25	25
Loss by washing, % max.	3.0	3.0	3.0
Thin and elongated pieces, % max.	10	10	15
Soft particles, % max.	-	-	5
Crushed material, % max.	50	40	40