

Member of the SNC-Lavalin Group

Performance Based Practical Design/Data Driven Safety Analysis

Module 1 – What is DDSA

Session Starts at 10 am

#### Welcome





#### Instructors



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## Agenda

- Welcome
- Intro from MDOT to set the stage
- Intro to DDSA What is DDSA; HSM vs traditional safety analysis vs systemic
- Intro to DDSA examples
- Break
- The HSM What is the HSM and how it works, HSM performance measures, and examples
- HSM examples
- Wrap-up





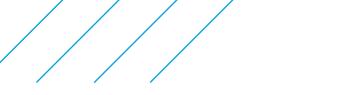
## Intro from MDOT





#### Introduction to DDSA







# Data Driven

# Safety Analysis





#### What is DDSA?

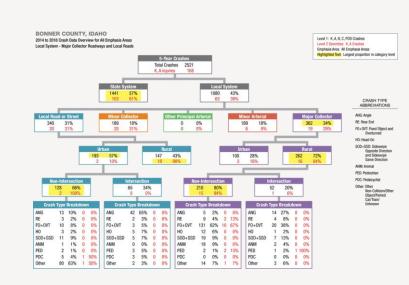


Using tools to analyze crash and roadway data to predict the safety impacts of highway projects allows agencies to target investments with more confidence and reduce severe crashes on the roadways.



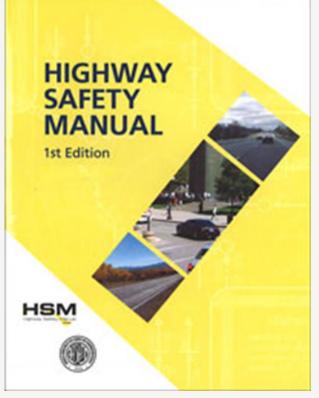
#### Safety Data Analysis

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Crash Data Collection

Spot vs. Systemic



**Predictive/HSM** 



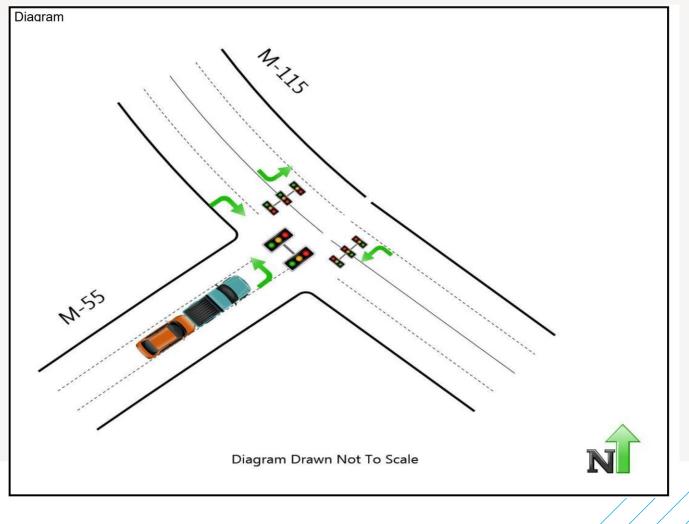
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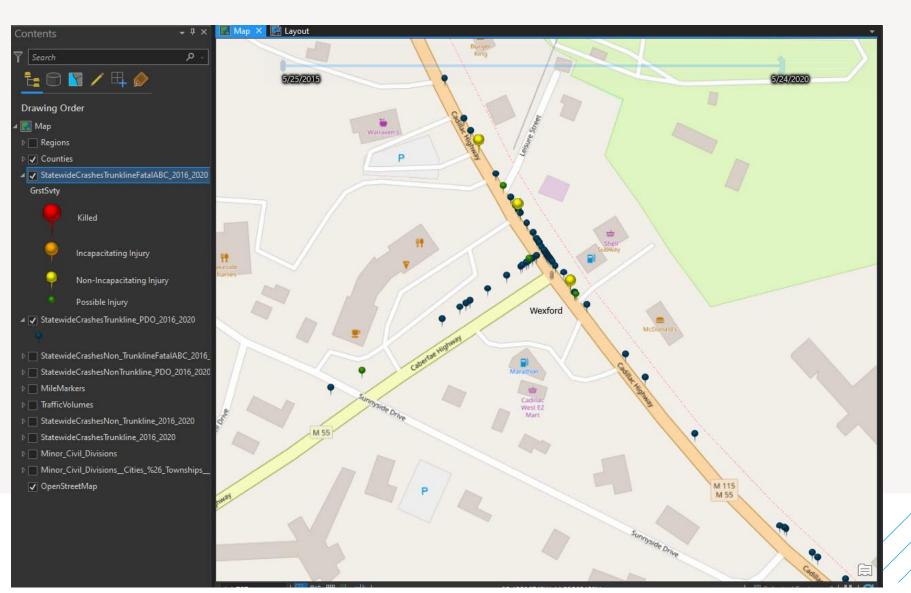




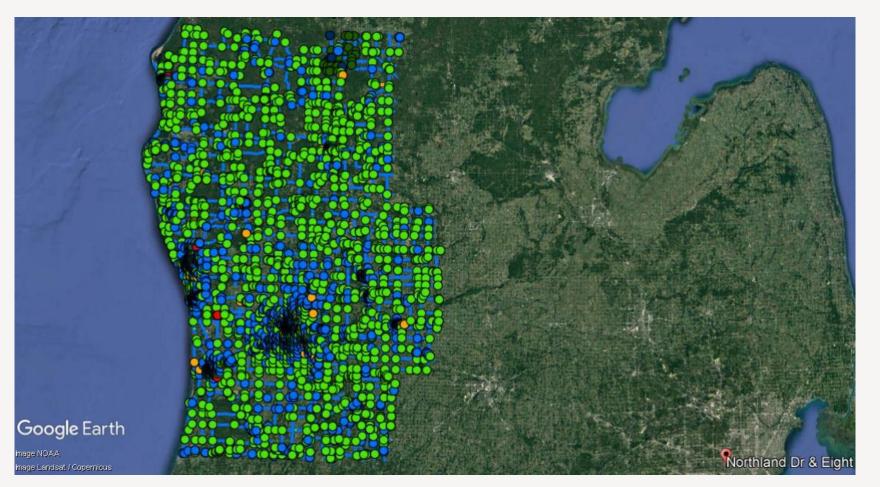
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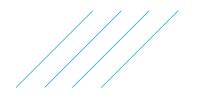
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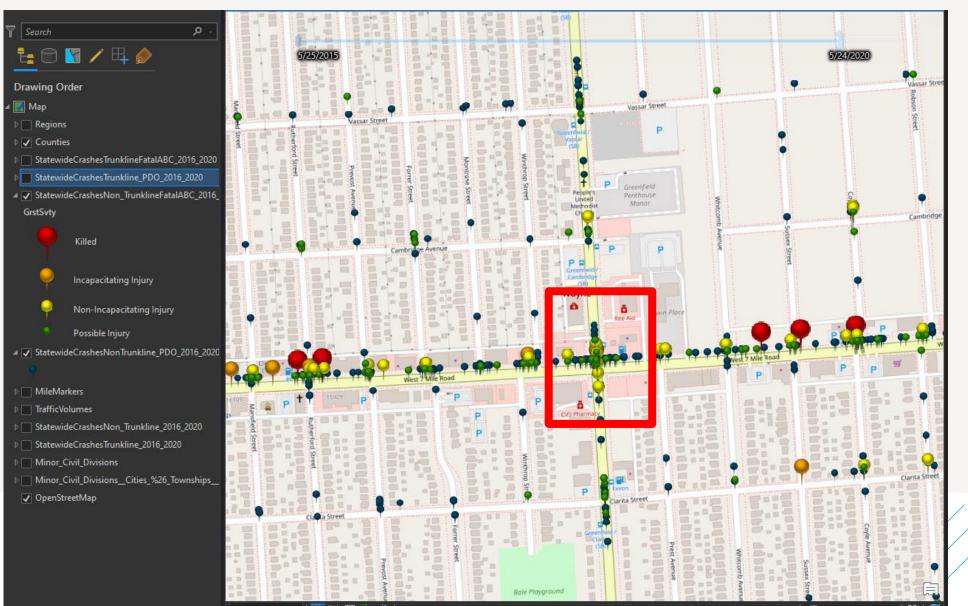
# Spot vs. Systemic





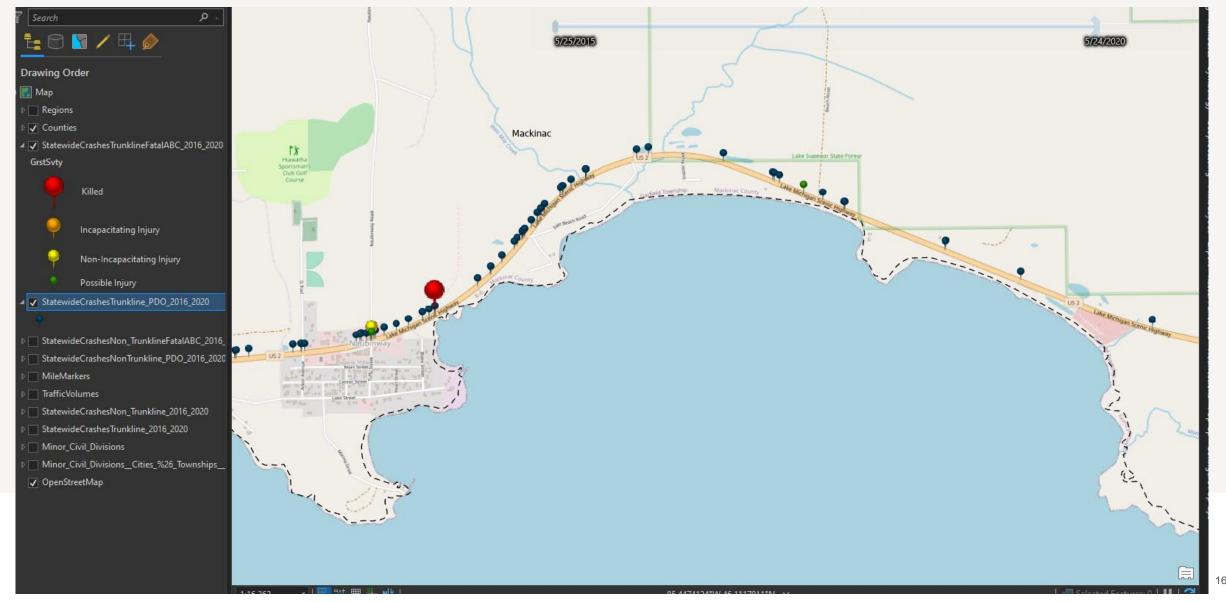
#### **Clusters of Traffic Crashes**

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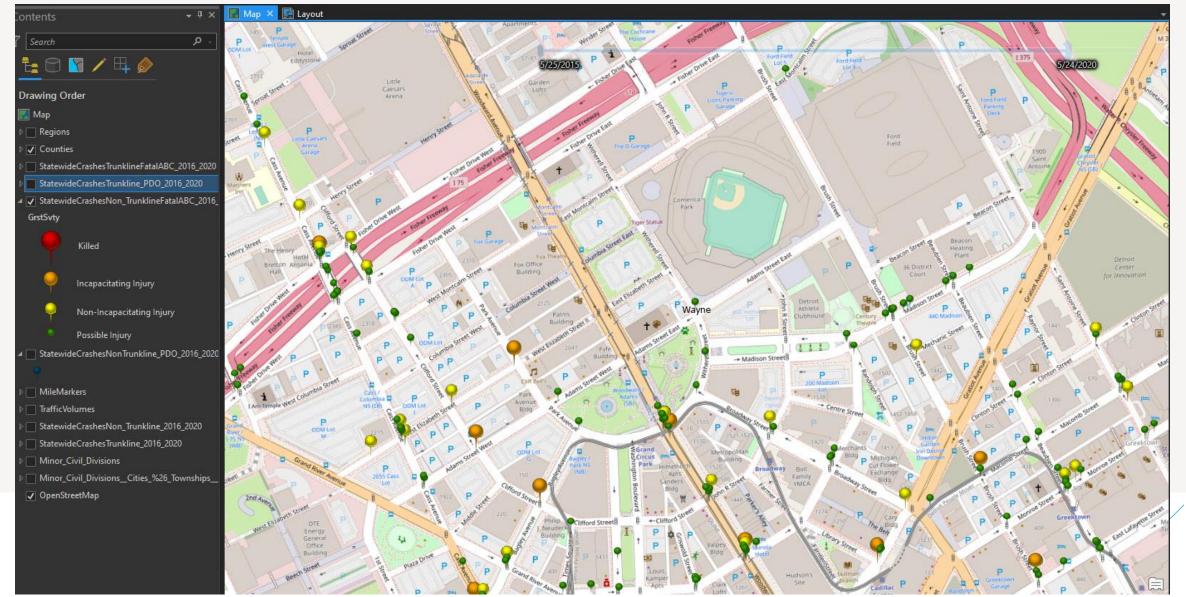


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#### **Distributed Traffic Crashes**



#### **Distributed Traffic Crashes**





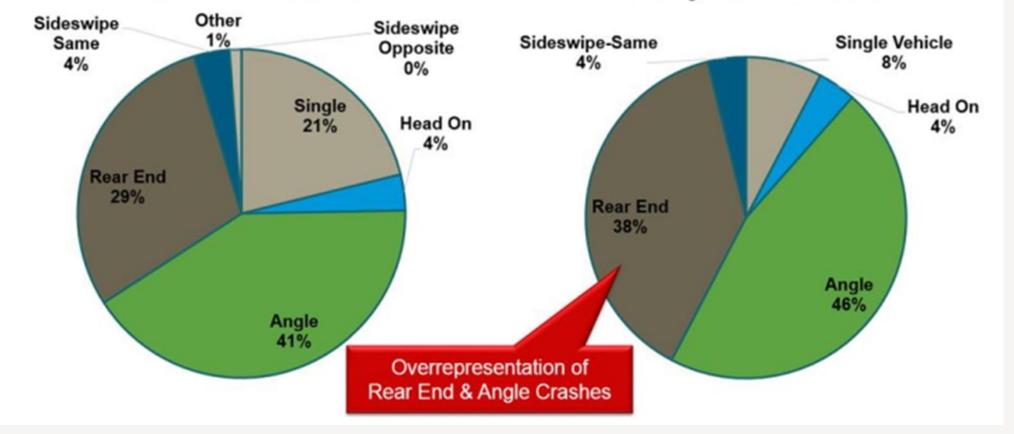
# Clusters of Crashes → Spot

# Distributed Crashes → Systemic





#### **Spot Analysis**



#### Similar Intersections

Study Intersection



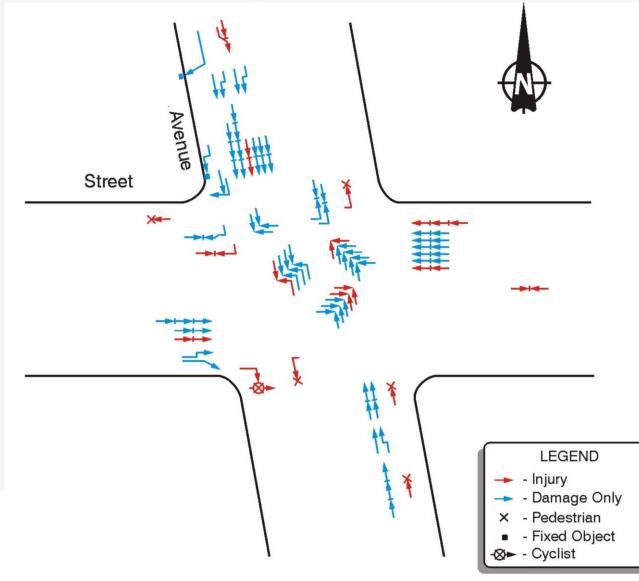


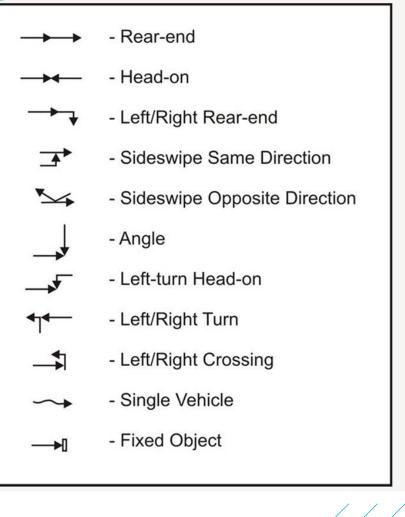
### **Spot Analysis**

Inters	ection Details	Traffic V	/olumes	Tra	ffic Crash	nes	Crash Type Distribution								
Minor	Configuration	Major	Minor	FI	PDO	тот	Single	Head On	Angle	Rear End	S.S. Same	S.S. Opp.			
Minor 1	Three Leg Stop	17,600	2,500	3	6	9	56%	0%	33%	11%	0%	0%			
Minor 2	Three Leg Stop	17,600	2,500	3	7	10	10%	20%	40%	20%	10%	0%			
Minor 3	Three Leg Stop	17,600	2,500	6	7	13	8%	0%	77%	15%	0%	0%			
Minor 4	Four Leg Signal	20,250	5,650	27	58	85	4%	2%	31%	56%	4%	4%			
Minor 5	Three Leg Stop	19,900	1,250	1	5	6	0%	33%	33%	17%	0%	17%			
Minor 6	Four Leg Signal	19,050	7,640	13	75	88	3%	2%	35%	50%	9%	0%			
Minor 7	Four Leg Stop	18,200	800	2	17	19	5%	0%	68%	11%	11%	5%			
Minor 8	Four Leg Stop	18,200	1,250	11	26	37	3%	3%	78%	11%	5%	0%			
All Corridor	Intersections	18,550	3,011	66	201	267	6%	3%	44%	39%	6%	2%			

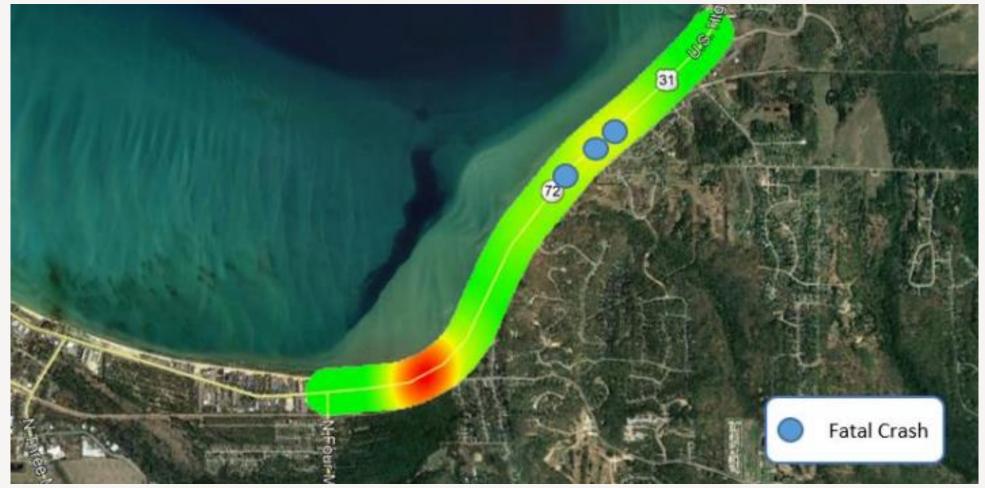


#### Spot Analysis – Collision Diagram

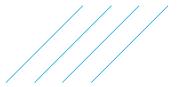




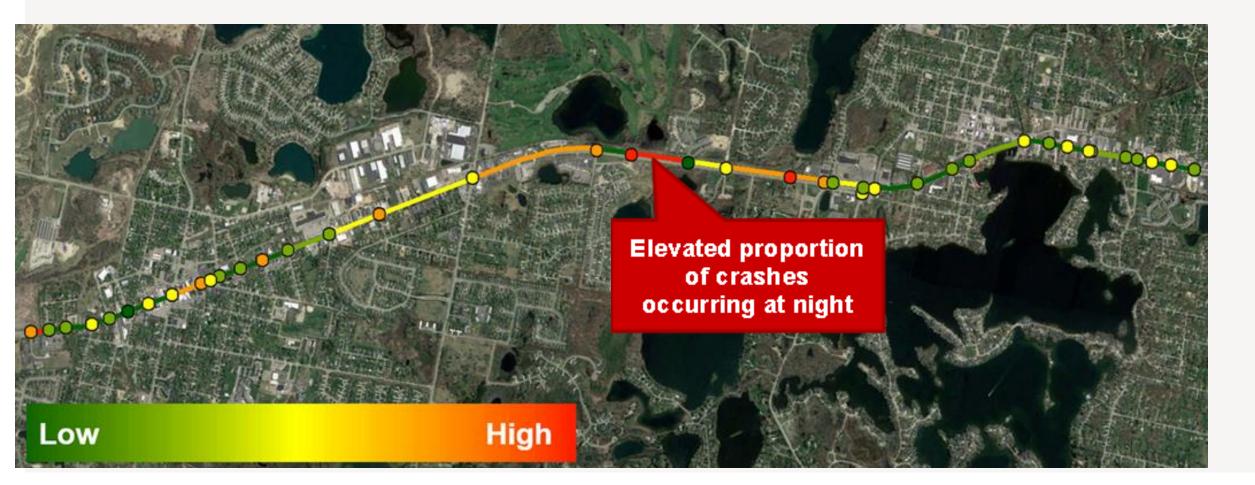
### **Spot Analysis**







### **Spot Analysis**





#### Example Spot Improvement



#### **Example Spot Improvement**





#### **Systemic Analysis**

#### Table 4: Percentage Distribution of Lane Departure Crashes by County, 2010-2014

Location	Crashes	Fatalities	A-injuries	K&A
Alger	34%	100%	63%	69%
Baraga	a 24% 100%		54%	61%
Chippewa	27%	67%	58%	59%
Delta	16%	40%	50%	49%
Dickinson	14%	75%	41%	45%
Gogebic	32%	50%	58%	57%
Houghton	25%	82%	47%	51%
Iron	23%	60%	54%	55%
Keweenaw	40%	100%	57%	64%
Luce	26%	71%	71%	71%
Mackinac	30%	70%	60%	61%
Marquette	24%	33%	41%	40%
Menominee	18%	58%	53%	54%
Ontonagon	20%	50%	52%	52%
Schoolcraft	23%	100%	63%	68%
Upper Peninsula	23%	62%	53%	54%



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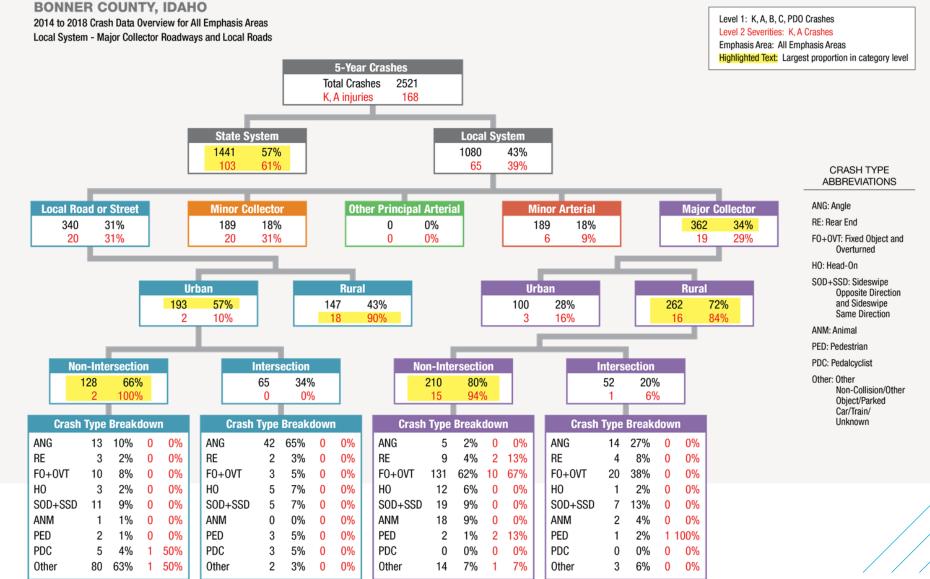
#### Systemic Analysis - Comparisons

Fatal and		ere Inju cent b	-		•	07-20	11)			
Emphasis Area		ewide 592 mi		tate 486 mi		unty 38 mi	City, Town, Village 76,735 mi			
Total Fatal/Serious Injury	100%	63,443	31%	19,819	10%	6,572	45%	28,597		
Pedestrian	19%	11,786	9%	1,860	6%	421	28%	8,122		
Bicycle	5%	3,390	3%	518	3%	187	8%	2,414		
Heavy Vehicle	5%	3,123	6%	1,266	4%	234	4%	1,051		
Road Departure	26%	16,668	30%	5,985	44%	2,892	18%	5,128		
Intersection	41%	25,791	25%	5,033	30%	1,957	64%	18,270		
Head-on and Sideswipe	5%	3,071	7%	1,439	7%	490	3%	887		





#### Systemic Analysis – Crash Tree



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#### Systemic Improvements – Rumble Strips







### Systemic Improvements - Delineation





#### **Systemic Strategies**

#### **Lane Departure**

1. Re-grading side slopes to 1:4, or flatter, to eliminate the need for guardrail

2. Guardrail improvements (SWA Funding eligible only)

3. Fixed object removal including clear zone widening, tree removal

4. Extending or modifying culvert ends to eliminate a fixed-objects in the clear zone 5. High-friction surface treatment (multi-location throughout Region)

6. Installing impact attenuators where one does not currently exist

7. Installing delineators as laid out in Standard Plan R-127

8. Installing channelization: quick curb, access management (right in/right out, etc.)

 Installing curve warning signs: chevrons, target arrows with reflective sign post strips

10. Eliminate edge drop-offs/rutting using Safety Edge installation

11. Construct centerline or shoulder rumble or mumble strips including widening shoulders to accommodate installation

12. Widen shoulders to decrease lane departure crashes





## **Systemic Strategies**

#### Intersections

1. Improvements to sight vision corners: tree/shrub removal, minimal site grading

2. Reflective sign post strips for horizontal alignment signs and /or stop, stop ahead, yield, or yield ahead signs.

3. Signing treatments for All WayStop and Cross Traffic Does NotStop Conditions as per SIGN-145-A

#### **Pedestrians**

1. Road Diets- Restriping only with no pavement overlays or reconstruction. (Form 1629 still needs to be followed)

2. Pedestrian Refuge Islands

3. Special Emphasis Pedestrian Crosswalk Markings as per PAVE-945

4. Rectangular Rapid Flashing Beacon (RRFB) – Approval per the Crosswalk Guidance Document 5. Pedestrian Hybrid Beacon (PHB) – Approval per the Crosswalk Guidance Document

6. Gateway Treatment as per the R1-6 User Guide





#### Five Minute Break



## Predictive Analysis/HSM





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#### **Issues with Traditional Crash Analysis**

HSM Addresses:

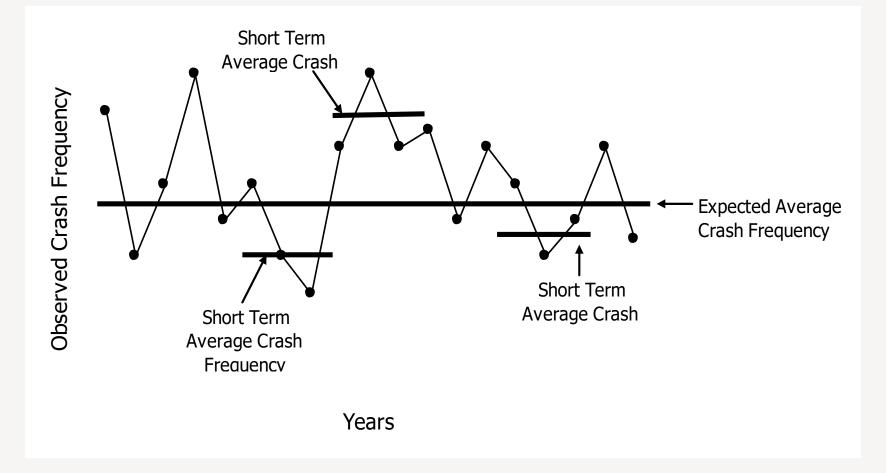
- Quality & accuracy
- Reporting thresholds
- Frequency-severity
- Differences between jurisdictions
- Randomness and change

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Hospital								Ambula	108							
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GVWR/GCWR					Vehicle (	Configuration		-	Cargo Body		S O X edical Card	0.01	Hazardo.	us Material	ID #	ľ
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Damaged Property						Publ	0	Dwner & I	Phone			-				=
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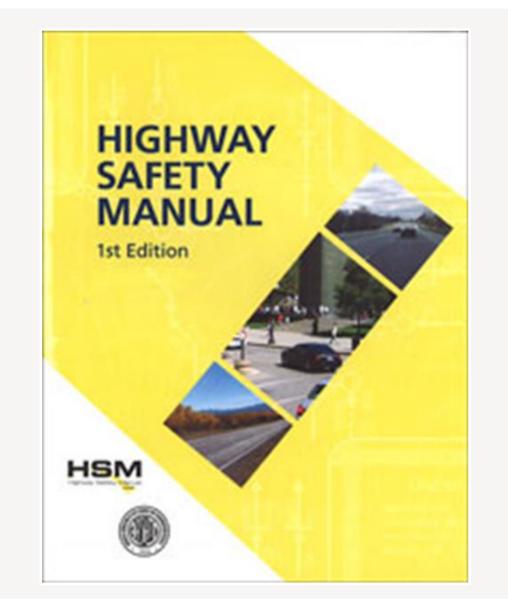
#### Natural Variability in Crash Frequency





## **Highway Safety Manual**

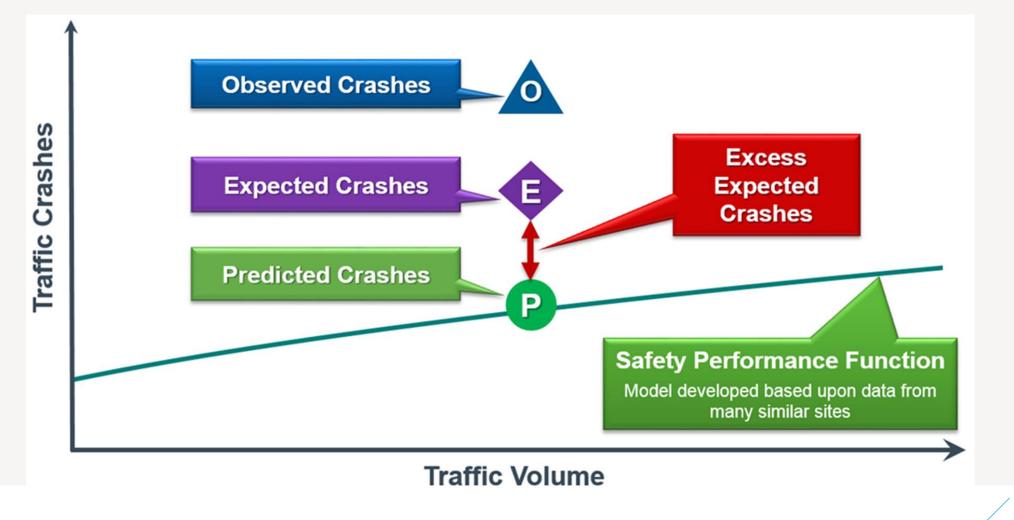
- Predictive modeling (safety performance functions)
- Network screening
- Scenario analysis





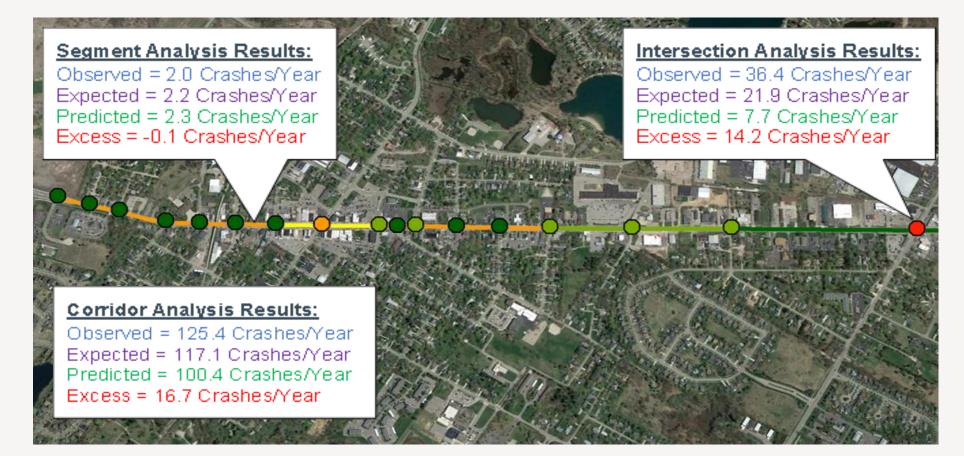


#### **HSM Performance Measures**



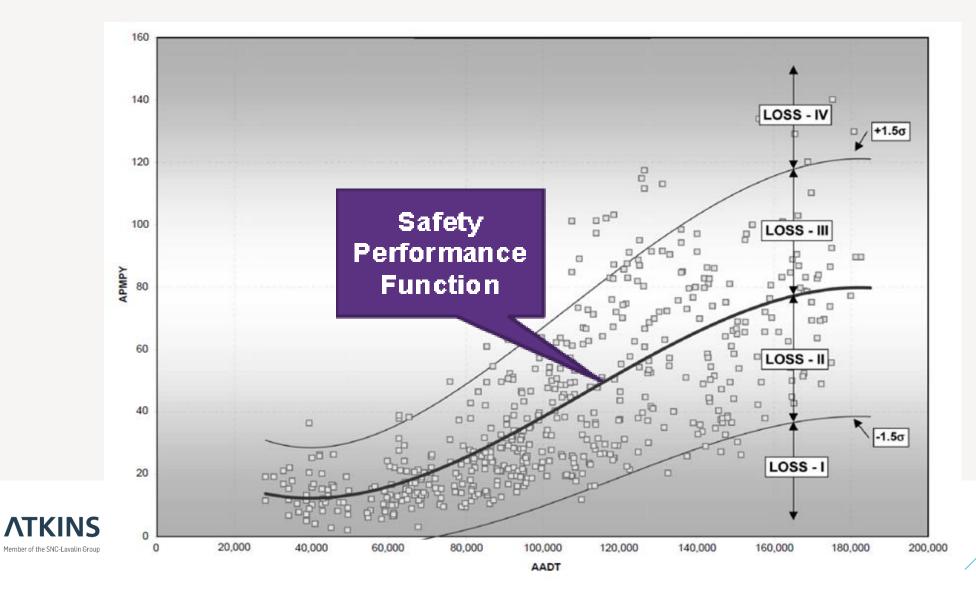


### **HSM** Analysis





#### Level of Service Safety



### Level of Service Safety

LOSS Category	Category Description
I	Indicates a low potential for crash reduction
II	Indicates a low to moderate potential for crash reduction
III	Indicates a moderate to high potential for crash reduction
IV	Indicates a high potential for crash reduction





# Applying DDSA on MDOT Projects

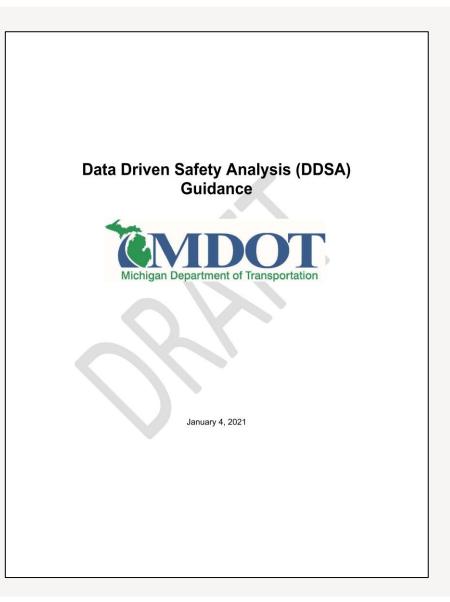




# **MDOT DDSA Guidance**

#### **Areas of Application**

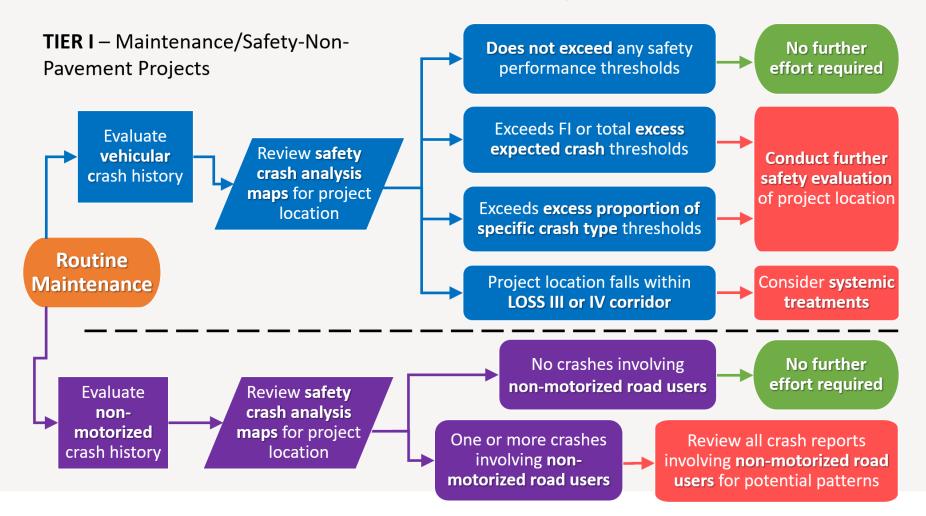
- Project development safety analysis
- Design Exceptions/Design Variances
- Alternative analysis as part of National Environmental Policy Act (NEPA)
- Interstate Access Requests
- Performance Based Practical Design (PBPD)





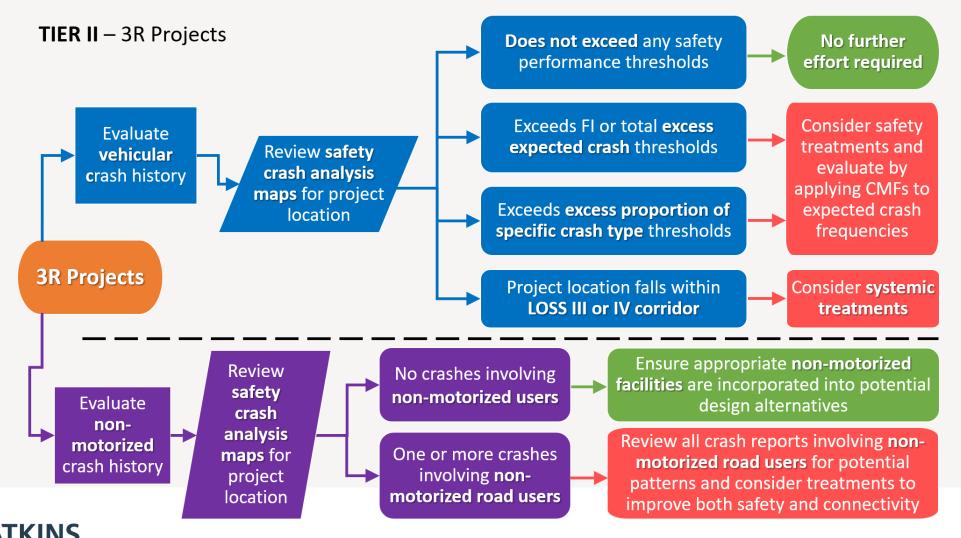


### Tier I – Maintenance/Safety Non-Pavement



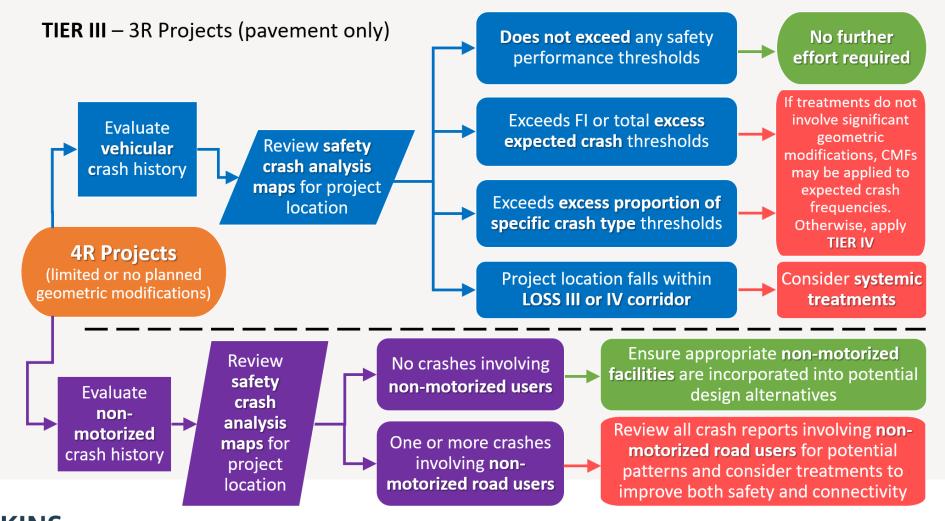


### Tier II – 3R Projects



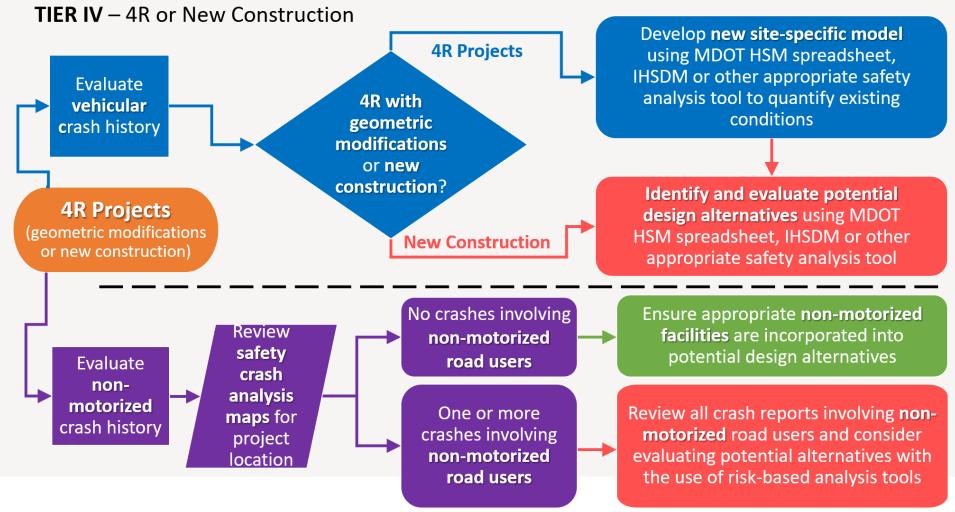
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### Tier III – 3R (pavement only)



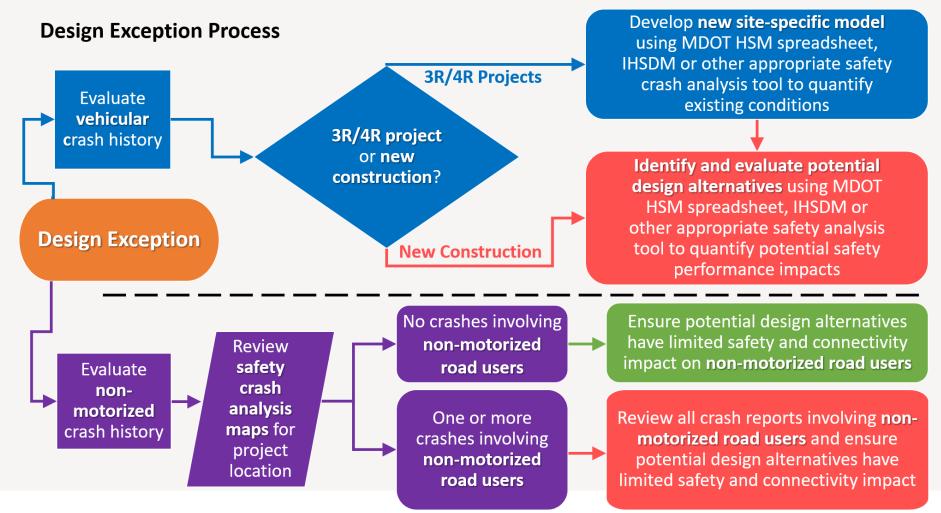


### Tier IV – 4R or New Construction





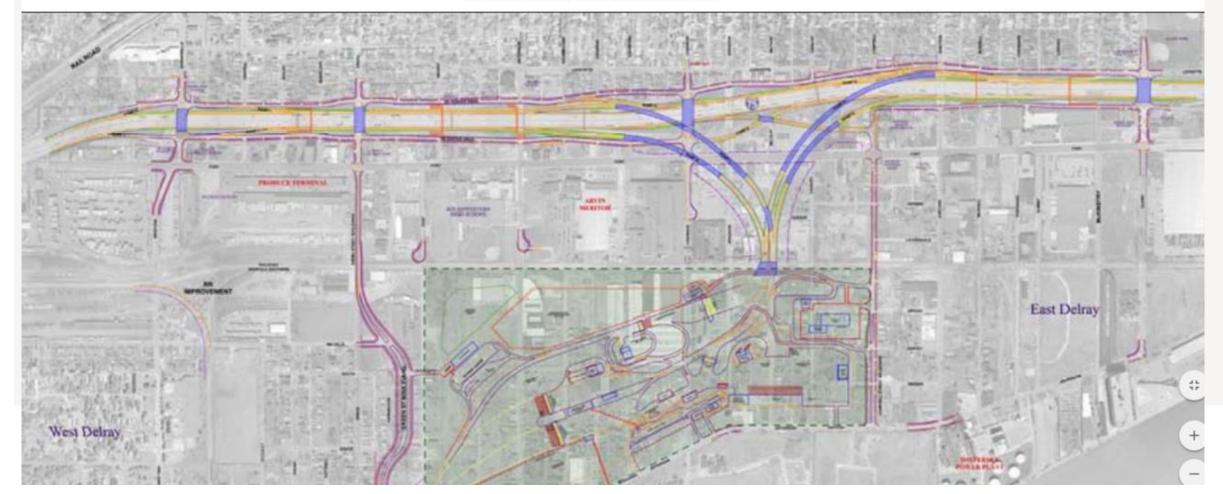
### **Design Exception Process**





### Upgrade to Existing vs. New Construction

Figure 2-15 Preferred Alternative Detroit River International Crossing Study



## Upgrade to Existing vs. New Construction

Project Type	Type of Improvement	HSM Performance Measure
Maintenance	Upgrade to existing	Excess expected crashes
3R	Upgrade to existing	Excess expected crashes
3R (Pavement)	Upgrade to existing	Excess expected crashes
4R	New construction	Predicted crashes
Design exception	Upgrade to existing	Excess expected crashes
Design exception	New construction	Predicted crashes



### Case Studies





#### Case Study – US-31 in Grand Traverse County



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#### Case Study – US-31 in Grand Traverse County

HSM Analysis for Intersections

Inte	rsection	Pred	icted Cra	ashes	Exp	ected Cra	ashes	Excess Crashes per Year			
Major	Minor	FI	PDO	Total	FI	PDO	Total	FI	PDO	Tot	
US-31	Five Mile Road	0.54	2.22	2.75	0.12	3.54	3.66	-0.42	1.32	0.91	
US-31	Holiday Road	0.10	1.27	1.37	0.47	6.99	7.46	0.37	5.72	6.09	
Overall		0.64	3.49	4.12	0.59	10.53	11.12	-0.05	7.04	7.00	



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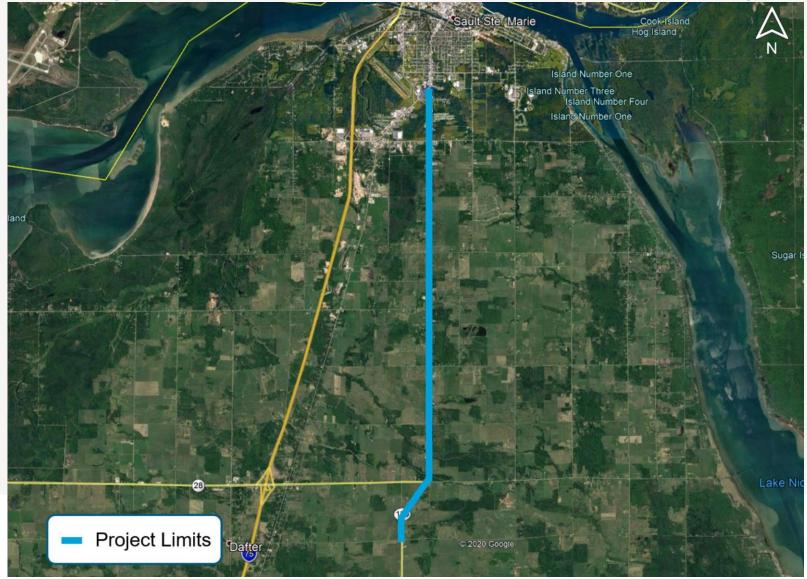
#### Case Study – US-31 in Grand Traverse County

**HSM** Analysis for Segments

	Section	Predi	cted Cr	ashes	Expe	cted Cra	ashes	Excess Crashes per Year			
Road	<b>Cross-Section</b>	FI	PDO	Total	FI	PDO	Total	FI	PDO	Tot	
US-31	Four-Lane Multi-Vehicle	5.58	33.48	39.06	2.59	7.38	9.97	-2.99	-26.1	-29.09	
US-31	Five-Lane Multi- Vehicle	5.46	21.6	27.06	1.46	6.17	7.63	-4	-15.43	-19.43	
US-31	Four-Lane Single Vehicle	0.72	7.41	8.13	0.88	8.90	9.78	0.16	1.49	1.65	
US-31	Five-Lane Single-Vehicle	0.57	3.58	4.25	0.78	7.23	8.01	0.21	3.55	3.76	
	Overall	12.33	66.17	78.5	5.71	29.68	35.39	-6.62	-36.49	-43.11	



#### Case Study – M-129 in Chippewa County







#### Case Study – M-129 in Chippewa County









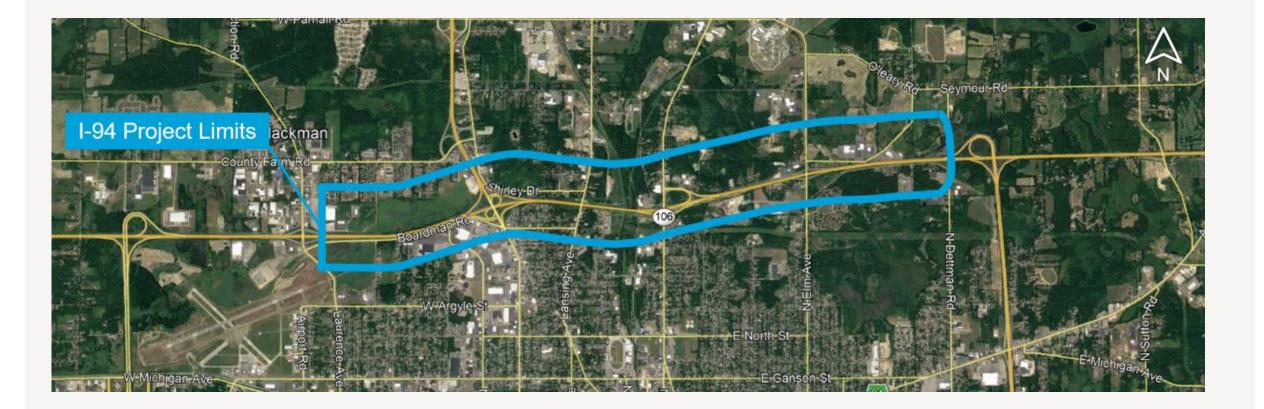
#### Case Study – M-129 in Chippewa County

			<b>Predicted Crashes</b>			ected Cra	shes	Excess Expected			
Location		FI	PDO	тот	FI	PDO	тот	FI	PDO	тот	
M-129 (10 Mile Rd to 3 Mile Rd)	Segment	1.84	5.42	7.26	1.46	3.12	4.58	-0.38	-2.3	-2.68	
M-129 (3 Mile Rd to I-75BS)	Segment	1.45	8.35	9.8	0.67	1.22	1.89	-0.78	-7.13	-7.91	
M-129 & M-28	Intersection	0.07	0.34	0.41	0.06	0.22	0.28	-0.01	-0.12	-0.13	
M-129 & 6 Mile Rd	Intersection	0.26	1.06	1.32	0.17	0.45	0.62	-0.09	-0.61	-0.7	
M-129 & 5 Mile Rd	Intersection	0.08	0.41	0.49	0.07	0.28	0.35	-0.01	-0.13	-0.14	
M-129 & 3 Mile Rd	Intersection	0.16	0.52	0.68	0.12	0.26	0.38	-0.04	-0.26	-0.3	
M-129 & I-75BS	Intersection	1.15	4.27	5.42	0.67	1.58	2.25	-0.48	-2.69	-3.17	
TOTAL		5.01	20.37	25.38	3.22	7.13	10.35	-1.79	-13.24	-15.03	



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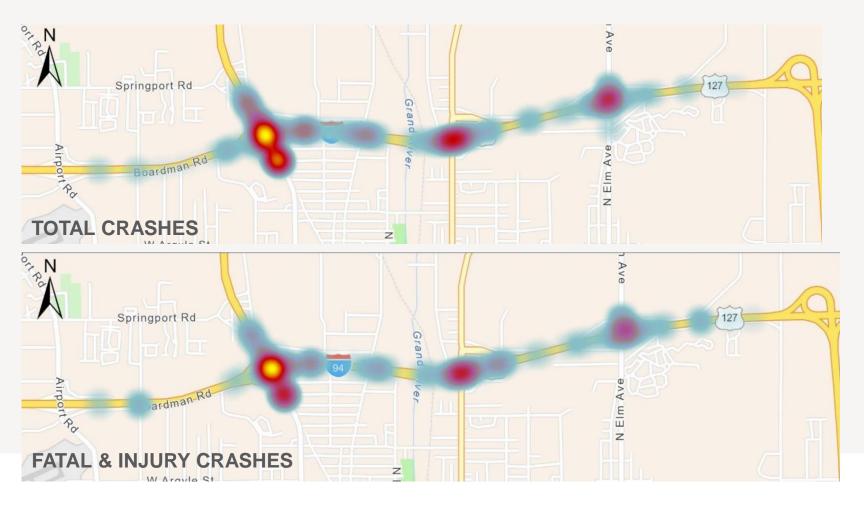
#### Case Study – I-94 in Jackson







#### Case Study – I-94 in Jackson





#### Case Study – I-94 in Jackson

		Predicted crashes without treatment			Predicted crashes with treatment			Change in predicted crashes		
Location			PDO	TOT	FI	PDO	ΤΟΤ	FI	PDO	TOT
I-94	Segment	49.22	91.89	141.11	37.80	62.30	100.10	11.42	29.59	41.01
US-127	Segment	7.12	12.21	19.33	10.59	17.29	27.88	-3.47	-5.08	-8.55
US-127/US-127BR/M-50 to EB I-94	Ramp	0.84	1.27	2.11	0.49	0.97	1.46	0.35	0.30	0.65
WB I-94 to US-127	Ramp	0.57	1.01	1.58	0.66	1.04	1.70	-0.09	-0.03	-0.12
US-127/US-127BR/M-50 to EB I-94	Ramp	0.43	0.60	1.03	0.22	0.35	0.57	0.21	0.25	0.46
EB I-94 to US-127BR/M-50	Ramp	0.30	0.40	0.70	0.14	0.20	0.34	0.16	0.20	0.36
EB I-94 to Elm Ave	Ramp	0.05	0.05	0.10	0.09	0.12	0.21	-0.04	-0.07	-0.11
Elm Ave to EB I-94	Ramp	0.09	0.10	0.19	0.10	0.15	0.25	-0.01	-0.05	-0.06
WB I-94 to Elm Ave	Ramp	0.06	0.09	0.15	0.11	0.14	0.25	-0.05	-0.05	-0.10
Elm Ave to WB I-94	Ramp	0.02	0.04	0.06	0.05	0.06	0.11	-0.03	-0.02	-0.05
Elm Ave & Carmen Dr	Intersection	0.65	1.97	2.62	0.18	0.47	0.65	0.47	1.50	1.97
Elm Ave & Rosehill Rd/Seymour Rd	Intersection	1.41	4.07	5.48	0.56	1.55	2.11	0.85	2.52	3.37
Elm Ave & Barrett Ln/Blake Rd	Intersection	0.18	0.47	0.65	0.65	1.97	2.62	-0.47	-1.50	-1.97
TOTAL		60.94	114.17	175.11	51.64	86.61	138.25	9.30	27.56	36.86



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